

Article **The Measurement of Shear Characteristics of Paddy Soil in Poyang Lake Area**

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Abstract: AbstractUnderstanding the shear characteristics of paddy fields is of great significance for the design of high-trafficability paddy wheels and to improve the trafficability of the tiller in paddy fields. Taking the paddy soil in Poyang Lake area of Jiangxi Province as the research object, a simple, self-designed bevameter was constructed to measure the apparent cohesion *c* and apparent internal friction angle *ϕ* of paddy soil with water content of 37% by weight using an annular shear plate. The effects of different shear speeds, shear grouser heights, and shear plate diameters on the measured *c* and φ were investigated. The measured values of *c* and φ were 1.89–2.35 kPa and 10.2–11.4[°] (deg), respectively. With the increase in shear speed, shear plate diameter, and grouser height, the value of *c* increased. Shear plate diameter and shear speed had small effect on *c*, whose change rate did not exceed 7.8%. However, shear grouser height had a greater influence on *c*, and the measured *c* increased by 24.5%. The influence of the three factors on φ followed the order: shear speed > shear plate diameter > shear grouser height, but the variation in φ did not exceed 1.2° (deg). The optimal operational conditions were: shear plate diameter of 250 mm with inner diameter of 150 mm, shear grouser height of 10 mm, with a shear speed of π/120 rad/s, and the apparent cohesion and apparent internal friction angle are 1887.8 pa and 11.06◦ (deg), respectively.

Keywords: shear strength; paddy soil; mechanical parameters; rotating shear

1. Introduction

Jiangxi Province is the main grain production region in China, ranking third in paddy sown area and total output. Improving the mechanization level of paddy field operations, and mechanizing paddy field cultivation, transplanting, fertilizing, and harvesting, can increase the production capacity $[1,2]$ $[1,2]$. The micro-tiller is the main power machine for paddy field cultivation, which is a light and compact mill type, enabling flexible operation [\[3\]](#page-8-2). However, when the wheel of the micro-tiller walks on the surface of the paddy soil, adhesion is low and running resistance and energy consumption is large due to severe sinking and slipping of the vehicle [\[4\]](#page-8-3). Therefore, it is important to improve the trafficability of the micro-tiller on the surface of the paddy soil [\[5](#page-8-4)[–7\]](#page-8-5).

Moreover, it is essential to understand the physical and mechanical properties of paddy soil for the design of reasonable soil-touching components. Previous studies have focused on synchronous side-deep fertilization of rice hole sowing [\[8](#page-8-6)[,9\]](#page-8-7) and mechanized rice direct seeding [\[10\]](#page-8-8), which are related to paddy soil preparation and formation [\[11\]](#page-8-9). However, the above studies did not mention the mechanical properties of surface and subsurface paddy soil, such as soil-shear strength [\[12\]](#page-8-10). The main parameters for evaluating soil-shear strength are cohesion c (Pa) and internal friction angle φ (measured in degrees) [\[13\]](#page-8-11), which are related by the Mohr–Coulomb law:

$$
\tau = c + \sigma \tan \varphi \tag{1}
$$

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where τ is the shear stress (Pa) and σ is the normal stress (Pa). It should be noted that when the soils are moist, c and φ in the Mohr–Coulomb equation represent the apparent cohesion and apparent internal friction angle due to the effect of pore water pressure [\[14\]](#page-8-12).

The methods for measuring soil shear strength are mainly divided into indirect and direct shear methods [\[15\]](#page-8-13). The indirect shear method usually estimates the shear strength of soil through soil compression and permeability tests [\[16,](#page-8-14)[17\]](#page-8-15), whereas the direct shear methods include translational methods and rotational methods. In the translational methods, the measured soil samples are placed into the direct shear box and four combined shear [\[18\]](#page-8-16), double direct shear, and constant deformation direct shear apparatus [\[19](#page-8-17)[,20\]](#page-9-0) are applied to determine the shear strength. Alternately, shear grousers installed on the track are applied as a shear component [\[21\]](#page-9-1) to obtain the shear strength. The rotational methods mainly use different types of rotational shear components to interact with the soil, such as hollow cylinders [\[22\]](#page-9-2), ring-shear devices [\[23](#page-9-3)[–25\]](#page-9-4), torsional shearing devices [\[26,](#page-9-5)[27\]](#page-9-6), vane shear test tools [\[28,](#page-9-7)[29\]](#page-9-8), or direct shear testers with rotational displacement and unlimited strain [\[30\]](#page-9-9). The shear strength of various soil types is calculated by recording parameters such as normal stress, torsional shear force, shear displacement or angle, and velocity.

Paddy soil is a typical soft ground type with obvious stratification; the soil cap is composed of the plough layer and the plow pan layer. The depth of the plough layer is generally 18–22 cm, with loose soil, whereas the plow bottom is generally 6–9 cm, with dense soil. When the water content of paddy soil is between 22% and 35% by weight, the rotation shear method is generally used to test the in situ shear strength of the plough layer [\[28](#page-9-7)[,31,](#page-9-10)[32\]](#page-9-11).

Therefore, a lightweight, portable bevameter with simple structure was designed herein to measure *c* and *ϕ* of surface and subsurface cohesive paddy soil with high water content. Subsequently, the effects of different shear speeds, shear grouser heights, and shear plate diameters on the shear mechanical parameters of paddy soil were investigated. The optimum test parameters were obtained according to the test results, and the apparent cohesion and apparent internal friction angle of paddy soil were measured under the test parameters.

2. Experiments and Methods

2.1. Sampling

Paddy soil was taken from the plough layer at a depth of 0–20 cm in $28.957370°$ N, 116.062260[°] E, Xinjian County, China. After drying and sieving, the physical parameters, including liquid and plastic limit, particle size distribution, and specific gravity (dry bulk density), were measured (Table [1\)](#page-1-0). The liquid limit represents the threshold water content of the plastic state and the fluid state for clay soil. When the water content of the soil reaches or exceeds the liquid limit, plasticity changes to fluidity. Furthermore, there is almost no connection force between soil particles [\[33](#page-9-12)[,34\]](#page-9-13). To improve the trafficability of the tiller operating in fluid soil and to understand the nature of the fluid soil, shear parameter tests were conducted on the fluid soil with a water content of 37% by weight, which is slightly above the liquid limit.

Table 1. Physical parameters of the tested soil.

2.2. Test Device

The overall structure of the simple bevameter is shown in Figure [1.](#page-2-0) A control cabinet was installed on the braced frame, and an electromagnet was installed at the end of the electric push rod. The drive motor and the torque sensor were connected and linked to the shear shaft, which was used to fix the shear plate by a flange through a connecting piece.

Figure 1. Drawing showing the structure of the simple bevameter. 1. electric push rod, 2. coupling bolt, 3. braced frame, 4. control cabinet, 5. shear plate, 6. shear shaft, 7. connecting piece, 8. torque sensor, 9. drive motor.

When the shear test was carried out, a computer was used to input instructions to the control cabinet. After receiving the instructions, the control cabinet controlled the electric push rod to drive the drive motor, the torque sensor, and the shear plate to move down along the three linear guides as a whole. When the shear plate contacted the soil, the power supply of the electromagnet used to connect the electric push rod and the drive motor was disconnected. Then, the drive motor, the torque sensor, and the shear plate as a whole fell on the soil under the action of the system weight of 6.2477 kg and the shear plate as a whole fell
on the soil under the action of the system weight of 6.2477 kg and the shear plate gravity. After 30 s, the drive motor rotated and drove the shear plate to shear the soil.

The structure of the shear plate is shown in Figure 2. The shear ring is connected to the small disc by four screws, and a flange is fixed on the small disc to connect to the shear shaft. Figure 3 shows the sche[ma](#page-2-2)tic diagram of the shear ring and the grousers. The diameters of the outer and inner rings of the shear ring installed on the six grousers are *D* and *d*, respectively. This experiment involves three different diameters of shear plates (A, B, and C) and three different grouser heights (h) . The specific sizes and total weights are shown in Table 2.

Figure 2. Structure 2. Society of the shear plate. 1. flange, 3. small disc, 3. second 3. shear ring. 1. shear ring. Figure 2. Structure of the shear plate. 1. flange, 2. small disc, 3. screw, 4. grouser, 5. shear ring.

fively, and *O* is the center. **Figure 3.** Schematic diagram of the shear ring, where *D* and *d* are outer and inner diameter, respec-

Table 2. Specific size and total weight m_1 values.

Shear Plate *^D* **(mm)** *^d* **(mm) Weight (kg)**

2.3. Testing Method **entitle.** Subsequently, the solid shear mechanical parameters under the solid parameters under different shear mechanical parameters under different shear mechanical parameters under the solid paramete

To investigate the most suitable conditions for field shear tests, the normal stress, which is the ground pressure on unit area of the shear plate, was taken as the independent variable. Subsequently, the soil shear mechanical parameters under different shear speeds $(π/360, π/180,$ and $π/120$ rad/s), different grouser heights, and different shear plate diameters were measured. The rotary till extend the specific preparation process was as follows: $\frac{1}{\sqrt{2}}$

The paddy soil shear test was carried out using a soil bin with a length of 5 m, a width of 1.2 m, and a height of 1.2 m. Prior to each test, the paddy soil was prepared by the rotary tiller. The specific preparation process was as follows: (1) adjusting the water content of the real decade of 0.5 m α paddy soil at 37% by weight; (2) preparing the soil at a depth of 20 cm and a speed of 0.5 m/s; (3) smoothing the surface using a scraper. Subsequently, the following steps were taken:

- (1) Increasing the vertical load to the set value and inserting the control instruction of the electric push rod to drive the shear plate downward. When the shear plate contacted the soil, the electromagnet was disconnected, and the shear plate fell into the soil under gravity.
- (2) Ensuring the grousers fall into the soil, waiting 30 s, as shown in Figure [4a](#page-3-1).
- (3) Starting the test and recording the test data. $\mathcal{L}(\mathbf{a}) = \mathbf{a} \cdot \mathbf{b}$ and controlling the pushed power supply and controlling the pushed power supply and controlling the pushed power supply and controlling the
	- (4) After the test, connecting the electromagnet power supply and controlling the push First the test, connecting the electromagnet power supply and controlling the push rod to connect with the shear component to pull the shear plate back to the initial position. Figure [4b](#page-3-1),c show the final state and soil deformation of the shear test. position. Figure 4b,c show the final state and soil deformation of the shear test.
- (5) Cleaning up the paddy soil adhered to the shear plate. (5) Cleaning up the paddy soil adhered to the shear plate.
- (6) Repeating the process three times under the same conditions. (6) Repeating the process three times under the same conditions.
- (7) Replacing the test conditions, repeating the soil preparation and steps (1)–(6). (7) Replacing the test conditions, repeating the soil preparation and steps (1)–(6).

(**a**) (**b**) (**c**)

Figure 4. Testing process, (a) initial stage, (b) shear stage, (c) soil deformation.

2.4. Shear Model

Janosi improved the Mohr–Coulomb law [\[35](#page-9-14)[–37\]](#page-9-15) and the modified equation is as follows:

$$
\tau = (c + \sigma \tan \varphi) \left(1 - e^{-j/j_0} \right) \tag{2}
$$

where *j* is the soil shear displacement(m) and j_0 is the shear stress-displacement curve modulus of the soil(m), which can be obtained by the relationship between soil shear stress and displacement (see Figure [5\)](#page-4-0).

Figure 5. Relationship between soil shear stress and displacement. **Figure 5.** Relationship between soil shear stress and displacement.

By solving Equation (3), the following formula is obtained:

The main part of the simple bevameter is the shear ring, as shown in Figure 6, The main part of the simple bevameter is the shear ring, as shown in Figure [6,](#page-4-1) where r_1 and r_2 are the radii(m). The ring is divided into several infinitesimal elements ABCD, where $d\theta$ is the angle of the differential element ABCD to the entire ring and dr is the width of the differential element. The torque $T(N·m)$ of the entire ring can be calculated by the calculus method (Equation (3)):

$$
T = \int_{r_2}^{r_1} \int_0^{2\pi} \tau r^2 d\theta dr \tag{3}
$$

Figure 6. Figure 6. Diagram of the shear ring section. Diagram of the shear ring section.

By solving Equation (3), the following formula is obtained:

$$
\tau = \frac{3T}{2\pi (r_1^3 - r_2^3)}\tag{4}
$$

Therefore, for the ring-shear device, by measuring the change in the torque acting on the center O point of the ring with the shear angle and the normal stress σ , combined with the Mohr–Coulomb law, c and φ of the soil can be determined.

Figure 6. Diagram of the shear ring section. **3. Data Processing**

The relationship between shear torque and time, obtained under different test conditions, was converted into the relationship between shear stress and angle. The torque was converted into the shear force, according to Equation (4), and the angle was obtained by multiplying the test time by the shear velocity. The normal stress σ of the test is a dependent variable and can be calculated by Equation (5):

$$
\sigma = \frac{(m_1 + m_2)g}{\pi (r_1^2 - r_2^2)}\tag{5}
$$

where m_1 is the total weight(kg) of the shear plate, grousers and system, m_2 is the added weight(kg), and *g* is the acceleration of gravity(m·s⁻²).

To introduce the data source in more detail, the test results of shear plate A, with a grouser height of 30 mm and a shear speed of $\pi/180$ rad/s, were selected. Figure [7](#page-5-0) shows the relationship between shear stress and shear angle under these conditions.

Figure 7. Relationship between shear stress and shear angle.

Figure 7. Relationship between shear stress and shear angle.

 $\mathcal{C}(\mathcal{F})$ and increases and their decreases. That, $\mathcal{F}(\mathcal{F})$ is restablined, indicating that where a certain angle is reached, the value of *τ* no longer changes with an increase in rotational As seen in Figure [7,](#page-5-0) the value of τ increases with the increase in shear angle, and its slope first increases and then decreases. Finally, it is horizontal, indicating that when a shear angle values.

With a simple transformation of Equation (2), the following formula is obtained:

$$
\tau = \tau_{\text{max}} (1 - e^{-\theta/\theta_0}) \tag{6}
$$

is the shear and is the shear stress–shear and $\frac{1}{2}$ and $\frac{1}{$ in degrees, deg) of the soil, which can also be [ob](#page-4-0)tained by the method shown in Figure 5. The values of *τ*_{max} and *θ*₀, as shown in Table [3,](#page-5-1) can be obtained by fitting the relationship
hatmoon the characterized at the characterized by fitting to *Frantian (C*) between the shear stress and the shear angle of the soil, according to Equation (6) . ting the relationship between the shear stress and the shear angle of the soil, according where θ is the shear angle and θ_0 is the shear stress–shear angle curve modulus($^{\circ}$ measured

Table 3. Values of τ_{max} and θ_0 .

τ_{max} (Pa)	2753	2926	3019	3128	3217	
θ_0 (deg)	7.489°	7.503°	7.919°	7.888°	7.311°	

Based on the results, when the shear speed was $\pi/180$ rad/s and the normal stress σ was 2803.19–5298.74 Pa, the value of θ_0 was 7.311–7.919° (deg). The value of τ_max obtained under different normal stress *σ* values was substituted into the Mohr–Coulomb law for fitting (Figure [8a](#page-6-0)). The coefficient of determination \mathbb{R}^2 was 0.98, and *c* and φ values were 2274.88 Pa and 10.26° (deg), respectively.

stress under different shear speeds, (c) Relation curve of normal stress-shear stress under different grouser heights, (**d**) Relationship between normal stress and shear stress under different shear
shear stress different grouser heights, (**d**) Relationship between normal stress and shear stress under different **Figure 8.** (**a**) Fitting results of shear stress to normal stress, (**b**) Relation curve of normal stress–shear plate diameters.

4. Results and Analysis

4.1. Influence of Shear Speed

4. Results and Analysis the shear plate diameter should not be too small [\[24\]](#page-9-16). The smaller the diameter of the shear plate, the greater the sinkage during shearing under the same load, which would impede To determine the influence of shear speed on the test results, the grouser height and shear stress is relatively small, leading to a large error. Thus, the shear plate A and the grouser height of 30 mm were selected for comparison of the soil shear strength under different shear speeds. The relation curve of the normal stress–shear stress is shown in
Eisene ^ok from which the class and interest of the streight line were obtained. Beach an $\frac{1}{2}$ and $\frac{1}{2}$ were calculated as shown in Table [4.](#page-6-1) To determine the influence of shear speed on the test results, the grouser height and the instrument cleaning and testing. When the grouser height is too small, the measured Figure [8b](#page-6-0), from which the slope and intercept of the straight line were obtained. Based on

Table 4. Values of *c* and φ under different shear speeds.

Based on Table 4, the value of *c* increased from 2177.4 to 2346.2 Pa with the increase Based on Table [4,](#page-6-1) the value of *c* increased from 2177.4 to 2346.2 Pa with the increase in shear speeds, with a variation of 7.75%, indicating that the shear speed had little effect on the value of *c*. The value of φ first decreased and then increased, with a variation of φ and φ first decline the value of φ first decreased and the highlifth of φ then the value of φ . Considering the measurement error and the subtle differences in each soil preparation, it can be concluded that the shear speed has little or no effect on the test results, consistent selected for the subsequent experiments. approximately 1.0° (deg), indicating that the shear speed also had little effect on the value with the literature [\[30\]](#page-9-9). Based on the in situ test, the shear speed of $\pi/120$ rad/s was

4.2. Influence of Grouser Height

To investigate the influence of grouser height on the soil shear strength, the shear plate A and a shear speed of $\pi/120$ rad/s were selected. The relation curve of normal stress–shear stress is shown in Figure [8c](#page-6-0). Table [5](#page-7-0) shows the values of apparent cohesion *c* and apparent internal friction angle *ϕ*, obtained according to the intercept and slope of the fitting line.

Table 5. Values of c and φ under different grouser heights.

Grouser Height (mm)	10		30	
c (Pa)	1887.8	2120.6	2346.2	
φ (deg)	11.06°	11.42°	11.25°	

Based on the results shown in Table [5,](#page-7-0) grouser height only slightly affected φ under the same shear plate and shear speed. The value of φ changed within 0.36 $^{\circ}$ (deg) with the change in grouser height. The value of *c* changed greatly from 1887.8 to 2346.2 Pa with increase in grouser height, with a change rate of 24.3%. However, it is usually considered advantageous for accurate results to use grousers that are as low as possible because the error produced by soil shear at the side edges decreases. Therefore, the test results obtained with a grouser height of 10 mm are closest to the actual values because failure occurs not only in the horizontal plane $(r_1 - r_2)$, but also in the inner and outer vertical cylindrical surfaces of the annulus [\[14\]](#page-8-12).

4.3. Influence of Shear Plate Diameter

To investigate the influence of shear plate diameter on the shear results, grouser height of 10 mm and shear speed of $\pi/120$ rad/s were selected. The relationship between normal stress and shear stress is shown in Figure [8d](#page-6-0), and c and φ were obtained as shown in Table [6,](#page-7-1) according to the intercept and slope of the fitting line.

Shear Plate			∼	
c (Pa)	1887.8	1994.5	1999.7	
φ (deg)	11.06°	10.25°	10.48°	

Table 6. Values of c and φ under different shear plate diameters.

Based on Table [6,](#page-7-1) at consistent shear grouser heights and shear speeds, the measured *c* increased gradually, with a small change rate, when the shear plate diameter decreased. Moreover, the variation of *ϕ* was only about 0.8◦ (deg) with the change in shear plate diameter. Thus, it can be inferred that the shear plate diameters had little effect on the measured results of *c* and *ϕ*. However, the larger the shear plate diameter, the smaller the sinkage caused by the flow of soil under the same load, facilitating subsequent instrument cleaning. Therefore, the choice of a shear plate with a larger diameter is beneficial to the test in the field experiment [\[11\]](#page-8-9). Based on these results, the optimal conditions were found to be: shear plate A with an outer diameter of 250 mm and an inner diameter of 150 mm, a grouser height of 10 mm, and a shear speed of $\pi/120$ rad/s.

5. Conclusions

The shear mechanical parameters of cohesive soil in the paddy plough layer, with a water content of about 37% by weight, were measured using a simple bevameter. The values of *c* and φ were 1887.8–2346.2 Pa and 10.2–11.4 \degree (deg), respectively. The results show that the shear speed and the shear plate diameters have little effect on the soil test results. When the shear speed is $\pi/360$ rad/s, the apparent cohesion and apparent internal friction angle are 2177.4 pa and 10.76 $^{\circ}$ (deg), respectively. When the shear speed is $\pi/120$ rad/s, the apparent cohesion and apparent internal friction angle are 2346.2 pa and 11.25 ° (deg) deg, respectively. The increase rates are 7.75% and 4.55%, respectively, and there is no significant change. The value of *c* changed greatly from 1887.8 to 2346.2 Pa with increase in grouser height, with a change rate of 24.3%. For in situ tests, it is recommended to select a shear plate with outer diameter of 250 mm and inner diameter of 150 mm, a shear grouser height of 10 mm, and a shear speed of $\pi/120$ rad/s, to obtain accurate results. Understanding the mechanical parameters of paddy soil can not only help evaluate the performance of conventional agricultural machinery in paddy fields, but also provide a soil parameter basis for drag reduction optimization of tillage components and soil tillage. Future work will focus on equations taking the grouse height into account.

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