



Article Explicit Solution to Large Deformation of Cantilever Beam by Improved Homotopy Analysis Method II: Vertical and Horizontal Displacements

Yinshan Li, Xinye Li *, Chen Xie and Shuhao Huo

School of Mechanical Engineering, Hebei University of Technology, Tianjin 300401, China; yinshanli@hebut.edu.cn (Y.L.); chenxiehebut@163.com (C.X.); hsh1993126@outlook.com (S.H.) * Correspondence: xylihebut@163.com

Abstract: Explicit solutions to vertical and horizontal displacements are derived for large deformation of a cantilever beam under point load at the free end by an improved homotopy analysis method (IHAM). Quadratic and cubic nonlinear differential equations are adopted to construct more proficient nonlinear equations for vertical and horizontal displacements respectively combined with their currently available nonlinear displacement equations. Higher-order nonlinear iterative homotopy equations are established to solve the vertical and horizontal displacements by combining simultaneous equations of the constructed nonlinear equations and the auxiliary linear equations. The convergence range of vertical displacement is extended by the homotopy-Páde approximation. The explicit solutions to the vertical and horizontal displacements are in favorable agreements with the respective exact solutions. The convergence ranges for a relative error of 1% by the improved homotopy analysis method for vertical and horizontal displacements increases by 60% and 7%, respectively. These explicit formulas are helpful in practical engineering design for very slender structures, such as high-rise buildings and long bridges.



Citation: Li, Y.; Li, X.; Xie, C.; Huo, S. Explicit Solution to Large Deformation of Cantilever Beam by Improved Homotopy Analysis Method II: Vertical and Horizontal Displacements. *Appl. Sci.* **2022**, *12*, 2513. https://doi.org/10.3390/ app12052513

Academic Editor: Angelo Luongo

Received: 12 December 2021 Accepted: 6 January 2022 Published: 28 February 2022

Publisher's Note: MDPI stays neutral with regard to jurisdictional claims in published maps and institutional affiliations.



Copyright: © 2022 by the authors. Licensee MDPI, Basel, Switzerland. This article is an open access article distributed under the terms and conditions of the Creative Commons Attribution (CC BY) license (https:// creativecommons.org/licenses/by/ 4.0/). **Keywords:** improved homotopy analysis method; large deformation of cantilever beam; strong nonlinearity; vertical displacement; horizontal displacement

1. Introduction

In view of the shortage of land worldwide, especially in metropolitan areas, high-rise building is a prioritized land-efficient architectural form developed more than 100 years ago. High-rise building is the product of the comprehensive application of modern sciences and engineering technologies. Smooth construction and sustainability of this new structural form depends on more accurate calculation theory and method than those for non-high-rise structures in light of that the extrusion of high-rise building into space could be as high as 800 m plus, and slight artefacts could be exaggerated into intractable unpredictability. Since the 21st century, beam-like structures have achieved overwhelming popularity, such as in high-rise buildings [1,2], long-span bridges [3,4], robots and aerospace; however, analysis methods for this form of structure are still those conventional Euler-Bernoulli laws for large deflection beams, which contain implicit integral to hinder general engineering application. An explicit higher-resolution analysis method is expected to tackle the high nonlinearity in high-rise buildings or long bridges.

The general theory of elastic deflection is called Elastica. To solve a general problem of elastic large deflection, the equivalent load, deflection and longitudinal displacement can be expressed by an analytic expression of elliptic integrals with free-end rotation angle as parameters [5]; indeed, the results have been reported extensively in the classical works of elastic mechanics [6–8]. For a cantilever beam subject to point load, the solution to this elastic problem is still an implicit solution with an elliptic integral.

In this study, the high-rise building is simplified as a large deformation of a cantilever beam subject to point load at the free end, for example, of a loading extremity, as shown in Figure 1. Given that the traditional analytical methods for solving nonlinear equations such as perturbation [9], methods introduced in [10,11], Adomian decomposition method [12], and δ -expansion method [13], in addition to the elliptical integrals mentioned above, cannot provide accurate, convergent and simple solutions, we aim to derive an explicit and accurate expression for rotation angle θ_b , vertical displacement δ_v and horizontal displacement δ_h to minimize the potential unpredictability in the large deformation of the long cantilever-like building.



Figure 1. Large deformation of beam under point load at the free end.

Unlike the perturbation technique, the homotopy analysis method (HAM) is independent of any small/large physical parameters. HAM provides us great freedom to choose equation-type and solution expression of the high order approximation equations. A convergence-control parameter *h* is introduced into the series solutions to guarantee the convergence. HAM distinguishes itself from other analytic approaches to be applied to complicated problems with strong nonlinearity [14–27]. The Föppl–Von Kármán's plate equations were solved by HAM [28,29]. Especially, the steady-state resonant waves were firstly predicted by the HAM in theory [21,22,30,31], which were then confirmed experimentally [30].

Ji Wang et al. derived an explicit solution to the large deformation of a cantilever beam under point load at the free end with HAM [32]. However, the improficiency of this method is identified to be a low convergence rate due to the step-by-step iteration of solutions of linear differential equations to approximate the exact solutions of complex non-linear differential equations; moreover, it fails to serve explicit solutions of vertical and horizontal displacements for large deformation of the beam.

In our previous paper, "Explicit Solution to Large Deformation Cantilever Beam by Improved Homotopy Analysis Method I: ROTATION ANGLE", an improved homotopy analysis method (IHAM) was proposed. In IHAM, an auxiliary nonlinear operator ε is introduced to improve the convergence rate and extend the convergence range. The improved homotopy analysis method was used to obtain the explicit solution to the rotation angle for the large deformation of a cantilever beam under point load at the free end.

For small deflection, the horizontal displacement of the cantilever beam is neglected and only the deflection and rotation angle are calculated. For large deflection, horizontal displacement is signified not to be neglected. Based on the explicit solution to the rotation angle obtained in our previous paper, explicit expressions for the deflection and horizontal displacement are derived using the improved homotopy analysis method in this paper.

2. Improved Homotopy Analysis Method

2.1. Problem Description

The bending equations of a uniform cross-section beam with large deformation are [5-8]

$$\frac{\mathrm{d}\theta(s)}{\mathrm{d}s} = \frac{F}{EI}[l - x(s)] \tag{2.1a}$$

$$\frac{\mathrm{d}v(s)}{\mathrm{d}s} = \sin[\theta(s)] \tag{2.1b}$$

$$\frac{\mathrm{d}x(s)}{\mathrm{d}s} = \cos[\theta(s)] \tag{2.1c}$$

subject to boundary conditions

$$\theta(0) = 0, \ \theta'(L) = 0$$
 (2.2a)

$$v(0) = 0$$
 (2.2b)

$$x(0) = 0$$
 (2.2c)

where *s* is the arc-coordinate of the neutral axis of the beam; *x* denotes the horizontal coordinate from the fixed end; *L* represents the length of the beam; *F* stands for the point load at the free end; *EI* is the bending stiffness of the beam; θ specifies the rotation angle of cross-section of the beam; and *l* is the horizontal distance of two ends and is unknown.

Differentiating the equation with respect to *s* and then using the dimensionless variables $\xi = s/L$, $\alpha = FL^2/(EI)$, X = x/L, V = v/L, $\Theta_b = \theta_b/(\pi/2)$, $V_b = \delta_v/L$ and $U_b = \delta_h/L$, the original equations become

$$\theta''(\xi) + \alpha \cos[\theta(\xi)] = 0 \tag{2.3a}$$

$$V'(\xi) = \sin[\theta(\xi)] \tag{2.3b}$$

$$X'(\xi) = \cos[\theta(\xi)] \tag{2.3c}$$

subject to boundary conditions

$$\theta(0) = 0, \ \theta'(1) = 0$$
 (2.4a)

$$V(0) = 0$$
 (2.4b)

$$X(0) = 0$$
 (2.4c)

where the prime denotes the differentiation with respect to ξ .

For infinitesimal deformation, the below linear equations are sufficiently accurate

 θ

$$''(\xi) + \alpha = 0 \tag{2.5a}$$

$$V'(\xi) = \theta(\xi) \tag{2.5b}$$

$$X'(\xi) = 1 \tag{2.5c}$$

subject to boundary conditions

$$\theta(0) = 0, \ \theta'(1) = 0$$
 (2.6a)

$$V(0) = 0$$
 (2.6b)

X(0) = 0 (2.6c)

Accordingly, the solutions are

$$\theta(\xi) = \frac{\alpha}{2}(2-\xi)\xi \tag{2.7a}$$

$$V(\xi) = \frac{\alpha}{6}\xi^2(3-\xi)$$
(2.7b)

 $X(\xi) = \xi \tag{2.7c}$

The rotation angle, vertical and horizontal displacement of cross-section plane at end as denoted by $\theta_b = \theta(1)$, $V_b = V(1)$, $U_b = 1 - X(1)$, are calculated to be

$$\theta_b = \frac{\alpha}{2} \tag{2.8a}$$

$$V_b = \frac{\alpha}{3} \tag{2.8b}$$

$$U_b = 0 \tag{2.8c}$$

In our previous paper, "Explicit Solution to Large Deformation Cantilever Beam by Improved Homotopy Analysis Method I: ROTATION ANGLE", the improved homotopy analysis method was proposed to solve Equations (2.3a) and (2.4a), and a series of solutions to the rotation angle are derived as [27]:

$$\theta(\xi) = \theta_0(\xi) + \sum_{n=1}^{+\infty} \theta_n(\xi)$$
(2.9)

where

$$\theta_n(\xi) = \sum_{k=1}^{+\infty} a_k \xi^k, n = 1, 2, \cdots$$
(2.10)

In this paper, we apply the improved homotopy analysis method to derive the explicit expressions of deflection and horizontal displacement using Equations (2.3b), (2.4b), (2.3c), and (2.4c) on the basis of the derived explicit expressions of rotation angle in Equations (2.9) and (2.10).

2.2. Fundamentals of Improved Homotopy Analysis Method

For quick reference, the new improved homotopy analysis method (IHAM) is briefed. The IHAM is essentially different from the HAM. Table 1 compares the formulations of IHAM and HAM.

Table 1. Comparison of improved homotopy analysis method against homotopy analysis method.

Homotopy Analysis Method (HAM)	Improved Homotopy Analysis Method (IHAM)
Original N(ξ) = 0 and Auxiliary L(ξ) = 0	(1) Original N(ξ) = 0 and Auxiliary N ₀ (ξ) = 0 \Rightarrow New $\overline{N}(\xi; q) = 0$
$\Rightarrow \text{Higher order } \mathcal{H}(\xi;q) = 0$	(2) New $\overline{N}(\xi; q) = 0$ and Auxiliary $L(\xi) = 0$ \Rightarrow Higher order $\overline{\mathcal{H}}(\xi; q) = 0$

Notes: N—nonlinear operator, L—linear operator, N₀—auxiliary nonlinear operator, \overline{N} —new nonlinear operator, \mathcal{H} —homotopy operator, ξ —independent variable, q—embedding parameter.

Substantial difference lies in the different construction methods. HAM constructs a higher order nonlinear iterative homotopy differential equation by the selected linear differential equation and the original non-linear differential equation. However, IHAM builds up a higher-order nonlinear iterative homotopy differential equation using the selected linear differential equation, the selected simple non-linear differential equation and the original non-linear differential equation (See Table 1).

Generalized Equation (2.3b) or Equation (2.3c) have the form of

$$N[v(\xi)] = 0; \ v'(\xi) = f[w(\xi)], \xi \in [0, 1]$$
(2.11)

where in f(w) is a derivable function in compliance with the boundary conditions of $f_w(0) = c_0$, $f'_w(0) = c_1$, $\frac{1}{2!}f_w(0) = c_2$, $\frac{1}{3!}f''_w(0) = c_3$; $w(\xi)$ is the selected polynomial function of an independent variable in Equations (2.9) and (2.10). The boundary condition in Equation (2.4b) or Equation (2.4c) can be expressed as

$$v(0) = 0$$
 (2.12)

Note that it is unnecessary to assume the existence of any small or large parameters in Equation (2.11). Thus, the proposed approach is general.

Linearizing original Equation (2.11) yields

$$L[v(\xi)] = 0; \ v'(\xi) = c_0 + c_1 w(\xi), \xi \in [0, 1]$$
(2.13)

which complies to the boundary condition of

$$v(0) = 0$$
 (2.14)

We choose the simplest auxiliary nonlinear equations of

$$N_0[v(\xi)] = 0: \ v'(\xi) = c_0 + c_1 w(\xi) + c_2 \varepsilon_2 w^2(\xi) + c_3 \varepsilon_1 w^3(\xi), \ \xi \in [0, 1]$$
(2.15)

herein, c_0 , c_1 , c_2 and c_3 are undetermined coefficients; and $\varepsilon_1 \in [0, 1]$, $\varepsilon_2 \in [0, 1]$, $\varepsilon_1^2 + \varepsilon_2^2 \neq 0$. which satisfies the boundary condition of

$$v(0) = 0$$
 (2.16)

Detailed formulation for the improved homotopy analysis method could be found in our previous paper titled "Explicit Solution to Large Deformation Cantilever Beam by Improved Homotopy Analysis Method I: ROTATION ANGLE".

3. Vertical Displacement of Cantilever Beam by IHAM

3.1. Zero Order Deformation Equation

The nonlinear boundary value Equations (2.3b) and (2.4b) are then solved by IHAM to derive explicit form for V_b . To start with, $V(\xi)$ is expressed by power series of ξ , i.e.,

$$V(\xi) = \sum_{k=1}^{+\infty} b_k \xi^k \tag{3.1}$$

where b_k is a constant coefficient. Since Equation (2.7b) from linear equation is an appropriate initial guess, we choose

$$V_0(\xi) = \frac{\alpha}{6}\xi^2(3-\xi)$$
(3.2)

The equivalent nonlinear Equation (2.3b) becomes

$$V' - \theta + \frac{\varepsilon_1}{3!}\theta^3 = 0 \tag{3.3}$$

subject to boundary condition

$$V(0) = 0$$
 (3.4)

where ε_1 is the coefficient, and $\varepsilon_1 \in [0, 1]$.

When $\varepsilon_1 = 0$, Equation (3.3) becomes linear Equation (2.5b), so that

$$V' - \theta = 0 \tag{3.5}$$

When $\varepsilon_1 = 1$, Equation (3.3) becomes

$$V' - \theta + \frac{1}{3!}\theta^3 = 0$$
 (3.6)

Equation (3.6) is an approximate expression of the second-order Taylor expansion of sine function in view of that

$$\sin\theta \approx \theta - \frac{1}{3!}\theta^3 \tag{3.7}$$

According to the original Equation (2.3b) and the equivalent Equation (3.3), we define a nonlinear operator as

$$N_{v}[\nu(\xi;q),\psi(\xi,q),q,\varepsilon_{1}] = \frac{\mathrm{d}\nu(\xi;q)}{\mathrm{d}\xi} - \sin[q \cdot \psi(\xi,q)] - (1-q)\psi(\xi,q) + \frac{\varepsilon_{1}}{3!} \left(1-q^{3}\right) \left[\psi(\xi,q)\right]^{3}$$
(3.8)

where $q \in [0, 1]$ is an embedding parameter; $\nu(\xi; q)$, $\psi(\xi; q)$ are dependent on ξ and $q; \varepsilon_1$ is a control parameter to represent the convergent region, and $\varepsilon_1 \in [0, 1]$.

When q = 0, there is

$$N_{v}[\nu(\xi;0),\psi(\xi,0),0,\varepsilon_{1}] = \frac{d\nu(\xi;0)}{d\xi} - \psi(\xi,0) + \frac{\varepsilon_{1}}{3!}[\psi(\xi,0)]^{3}$$
(3.9)

If q = 1, it yields

$$N_{v}[\nu(\xi;1),\psi(\xi,1),1,\varepsilon_{1}] = \frac{d\nu(\xi;1)}{d\xi} - \sin[\psi(\xi,1)]$$
(3.10)

Letting

$$\mathcal{L}_{v}[\nu(\xi;q)] = \frac{\partial \nu(\xi;q)}{\partial \xi}$$
(3.11)

be an auxiliary linear operator, and $h \in [-1, 0)$ denotes a nonzero auxiliary parameter (called convergence-control parameter), a homotopy is then constructed as

 $\mathcal{H}_{v}[\nu(\xi;q),q] = 0$

$$\mathcal{H}_{v}[\nu(\xi;q),q] := (1-q)\mathcal{L}_{v}[\nu(\xi;q) - V_{0}(\xi)]$$
$$-qh_{1}\mathcal{N}_{v}[\nu(\xi;q),\psi(\xi,q),q,\varepsilon_{1}]$$
(3.12)

When q = 0, we have

$$\mathcal{H}_{v}[\nu(\xi;0),0] := \mathcal{L}_{v}[\nu(\xi;0) - V_{0}(\xi)]$$
(3.13)

If q = 1, there is

$$\mathcal{H}_{v}[\nu(\xi;1),1] := -h_{1}N_{v}[\nu(\xi;1),1,\varepsilon_{1}] = -h_{1}\left\{\frac{d\nu(\xi;1)}{d\xi} - \sin[\psi(\xi,1)]\right\}$$
(3.14)

Thus, by enforcing

we have a family of equations

$$(1-q)L_{v}[\nu(\xi;q) - V_{0}(\xi)] = qh_{1}N_{v}[\nu(\xi;q),\psi(\xi,q),q,\varepsilon_{1}]$$
(3.15)

subject to the boundary condition of

$$\nu(0;q) = 0 \tag{3.16}$$

When q = 0, we have

$$\nu(\xi;0) = V_0(\xi), \psi(\xi;0) = \theta_0(\xi)$$
(3.17)

When q = 1, Equations (3.15) and (3.16) are equivalent to the original Equations (2.3b) and (2.4b), provided that

$$\nu(\xi; 1) = V(\xi), \psi(\xi; 1) = \theta(\xi)$$
(3.18)

Thus, as *q* increases from 0 to 1, $\nu(\xi; q)$ evolves smoothly from the specified initial guess $V_0(\xi)$ to the solution $V(\xi)$ of Equations (2.3b) and (2.4b).

With reference to Equation (3.17), by expanding $\nu(\xi; q)$ into Taylor series of the embedding parameter q, we have

$$\nu(\xi;q) = V_0(\xi) + \sum_{n=1}^{+\infty} V_n(\xi)q^n$$
(3.19a)

$$\psi(\xi;q) = \theta_0(\xi) + \sum_{n=1}^{+\infty} \theta_n(\xi) q^n$$
(3.19b)

where

$$V_n = \left. \frac{1}{n!} \frac{\partial^n \nu(\xi;q)}{\partial q^n} \right|_{q=0}, \ \theta_n = \left. \frac{1}{n!} \frac{\partial^n \psi(\xi;q)}{\partial q^n} \right|_{q=0}$$

It is worth noting that the zero order deformation Equation (3.15) contains auxiliary parameters ε_1 , h_1 . Auxiliary function $H_1(\xi; q, \varepsilon_1)$ has the form of

$$H_1(\xi; q, \varepsilon_1) = -(1-q)\psi(\xi, q) + \frac{\varepsilon_1}{3!} (1-q^3) [\psi(\xi, q)]^3$$

Assuming that h_1 and ε_1 are properly chosen, so that Equation (3.18) is convergent at q = 1. Thus, we have

$$V(\xi) = V_0(\xi) + \sum_{n=1}^{+\infty} V_n(\xi)$$
(3.20a)

$$\theta(\xi) = \theta_0(\xi) + \sum_{n=1}^{+\infty} \theta_n(\xi)$$
(3.20b)

The governing equations of $V_n(\xi)$ can be deduced from the 0th-order deformation Equations (3.15) and (3.16).

3.2. High Order Deformation Equations

Substituting Equation (3.19) into Equation (3.15) and differentiating Equation (3.15) n times with respect to the embedding parameter q, then dividing by n! and setting q = 0, we have the n th-order deformation equation as below,

$$L_{v}[V_{n}(\xi) - \chi_{n}V_{n-1}(\xi)] = h_{1}R_{n}^{v}(V_{0}, V_{1}, \cdots, V_{n-1}; \theta_{0}, \theta_{1}, \cdots, \theta_{n-1})$$
(3.21)

Equation (3.21) is subject to boundary conditions of

$$V_n(0) = 0$$
 (3.22)

where

$$R_n^v = \left. \frac{1}{(n-1)!} \frac{\partial^{n-1} \mathcal{N}_v[\nu(\xi;q),\psi(\xi;q),q,\varepsilon_1]}{\partial q^{n-1}} \right|_{q=0}$$
(3.23)

and

$$\chi_n = \begin{cases} 0, n \le 1\\ 1, n > 1 \end{cases}$$
(3.24)

Combining Equations (3.8) and (3.23), we have

$$R_1^{\nu} = V'_0(\xi) - \theta_0(\xi) + \frac{\varepsilon_1}{6} \theta_0^3(\xi)$$
(3.25a)

$$R_{2}^{v} = V'_{1}(\xi) - \theta_{1}(\xi) + \frac{\varepsilon_{1}}{2}\theta_{0}^{2}(\xi)\theta_{1}(\xi)$$
(3.25b)

$$R_{3}^{v} = V'_{2}(\xi) - \theta_{2}(\xi) + \frac{\varepsilon_{1}}{2}\theta_{0}^{2}(\xi)\theta_{2}(\xi) + \frac{\varepsilon_{1}}{2}\theta_{0}(\xi)\theta_{1}^{2}(\xi)$$
(3.25c)

$$R_{4}^{v} = V'_{3}(\xi) - \theta_{3}(\xi) + \frac{1}{6}\theta_{0}^{3}(\xi) - \frac{\varepsilon_{1}}{6}\theta_{0}^{3}(\xi) + \frac{\varepsilon_{1}}{6}\theta_{0}^{3}(\xi) + \frac{\varepsilon_{1}}{6}\theta_{0}^{3}(\xi) + \varepsilon_{1}\theta_{0}(\xi)\theta_{1}(\xi)\theta_{2}(\xi)$$
(3.25d)

$$R_{5}^{v} = V'_{4}(\xi) - \theta_{4}(\xi) + \frac{1}{2}\theta_{0}^{2}(\xi)\theta_{1}(\xi) - \frac{\varepsilon_{1}}{2}\theta_{0}^{2}(\xi)\theta_{1}(\xi) + \frac{\varepsilon_{1}}{2}\theta_{0}^{2}(\xi)\theta_{4}(\xi) + \frac{\varepsilon_{1}}{2}\theta_{0}(\xi)\theta_{2}^{2}(\xi) + \frac{\varepsilon_{1}}{2}\theta_{1}^{2}(\xi)\theta_{2}(\xi) + \varepsilon_{1}\theta_{0}(\xi)\theta_{1}(\xi)\theta_{3}(\xi)$$
(3.25e)

These equations can be solved by symbolic calculation software such as Maple, Matlab, etc. Thereafter, the linear high-order deformation Equations (3.21) and (3.22) can be solved explicitly.

The solutions given by the IHAM contains auxiliary parameters ε_1 and h_1 , which control and adjust the convergence region and rate of IHAM solution series.

3.3. Vertical Displacement at Free end of Cantilever Beam

3.3.1. Accurate Solution of Vertical Displacement

From Equation (2.3), the transcendental equation used to solve V_b is as follows

$$V_b = 1 - \frac{2}{\sqrt{\alpha}} [E(\mu) - E(\phi, \mu)]$$
(3.26)

where

$$E(\mu) = \int_0^{\pi/2} \frac{\mathrm{d}t}{\sqrt{1 - \mu^2 \sin^2 t}}$$
(3.27a)

$$E(\phi,\mu) = \int_0^{\phi} \frac{dt}{\sqrt{1 - \mu^2 \sin^2 t}}$$
(3.27b)

herein $E(\mu)$ is complete elliptic integral of the second kind; $E(\phi, \mu)$ is the elliptic integral of the second kind; and

$$\phi = \arcsin\frac{1}{\sqrt{2}\,\mu} \tag{3.28}$$

$$\mu = \sqrt{\frac{1 + \sin \theta_b}{2}} \tag{3.29}$$

From Equation (2.8b), the linear solution of the vertical displacement at the free end is

$$V_b = \frac{\alpha}{3} \tag{3.30}$$

When the relative error $\Delta \leq 1\%$, the convergent region of the linear solution is

$$\alpha \in [0, 0.29], V_b \in [0, 0.097], 0 \le \delta_v \le 0.097L$$
 (3.31)

3.3.2. Control Parameter $\varepsilon_1 \in [0, 1]$ for Convergent Region

Combining Equation (2.3b) with Equation (3.3) gives

$$\sin\theta = \theta - \frac{\varepsilon_1}{3!}\theta^3 \tag{3.32}$$

When $\theta = \frac{\pi}{2}$, we have

$$1 = \frac{\pi}{2} - \frac{\varepsilon_1}{3!} \left(\frac{\pi}{2}\right)^3 \tag{3.33}$$

Accordingly, the optimal convergence parameter for the full coverage is derived as

$$\varepsilon_1 = \frac{24(\pi - 2)}{\pi^3}$$
(3.34)

When n = 2, h = -1, $\varepsilon = 8/\pi^2$, the 2nd-order approximation of V_b by IHAM is

$$V_b = \frac{1}{3}\alpha + 2.3159 \times 10^{-2}h_1\alpha^3 + 1.9048 \times 10^{-3}h_1\varepsilon_1\alpha^3 + 9.5238 \times 10^{-3}h_1^2\varepsilon_1\alpha^3 - 2.0430 \times 10^{-3}h_1\varepsilon_1\alpha^5$$
(3.35)

For example, the 2nd-order approximation of V_b with $h_1 = -1$, $\varepsilon_1 = 24(\pi - 2)/\pi^3$ is

$$V_b = \frac{1}{3}\alpha - 3.1575 \times 10^{-2}\alpha^3 + 1.8053 \times 10^{-3}\alpha^5$$
(3.36)

When the relative error $\Delta \leq 1\%$, the convergent region of the 2nd-order approximation by IHAM is

$$\alpha \in [0, 1.5], V_b \in [0, 0.41], 0 \le \delta_v \le 0.41L$$
 (3.37)

Table 2 shows 2nd order numerical solutions $V_b^{[n=2]}$ by IHAM with different values ε_1 compared with exact solutions V_b . The relative error in Table 2 is calculated by

$$\Delta = \frac{\left| V_b^{[n=2]} - V_b \right|}{V_b} \times 100\%$$
(3.38)

Table 2. 2st-order numerical solutions $V_b^{[n=2]}$ by IHAM ($h_1 = -1$) with different values ε_1 compared with exact solutions V_b .

а	Exact Solution	ε ₁ (ΗΑ	= 0 AM)	$\epsilon_1 = 24(\pi - 2)/\pi^3$ (IHAM)		$\varepsilon_1 = 1$ (IHAM)	
	V _b	V_b	Δ	V_b	Δ	V_b	Δ
0	0	0	0	0	0	0	0
0.1	0.033295	0.033310	0.0446%	0.033302	0.0194%	0.033301	0.0161%
0.2	0.066364	0.066481	0.176%	0.066414	0.0756%	0.066405	0.0623%
0.3	0.098991	0.099375	0.388%	0.099152	0.163%	0.099123	0.133%
0.4	0.13098	0.13185	0.669%	0.13133	0.272%	0.13126	0.219%
0.5	0.16214	0.16377	1.00%	0.16278	0.390%	0.16265	0.309%
0.6	0.19235	0.19500	1.38%	0.19332	0.504%	0.19310	0.389%
0.7	0.22148	0.22539	1.76%	0.22281	0.599%	0.22247	0.445%
0.8	0.24945	0.25481	2.15%	0.25109	0.658%	0.25060	0.461%
0.9	0.27621	0.28312	2.50%	0.27805	0.666%	0.27738	0.425%
1.0	0.30172	0.31017	2.80%	0.30356	0.611%	0.30269	0.322%
1.1	0.32598	0.33584	3.02%	0.32755	0.480%	0.32646	0.145%
1.2	0.34901	0.35998	3.14%	0.34993	0.264%	0.34861	0.115%
1.3	0.37082	0.38245	3.14%	0.37067	0.0427%	0.36911	0.461%
1.4	0.39147	0.40312	2.97%	0.38973	0.442%	0.38797	0.892%
1.5	0.41098	0.42184	2.64%	0.40714	0.933%	0.40521	1.40%
1.6	0.42941	0.43847	2.11%	0.42293	1.51%	0.42089	1.99%
1.7	0.44682	0.45289	1.36%	0.43717	2.16%	0.43510	2.62%

а	Exact Solution	ε ₁ (ΗΑ	= 0 AM)	$\epsilon_1 = 24(\alpha)$	$(\pi-2)/\pi^3$ AM)	ε ₁ (IHA	= 1 AM)
	V_b	V_b	Δ	V_b	Δ	V_b	Δ
1.8	0.46326	0.46494	0.361%	0.44997	2.87%	0.44800	3.30%
1.9	0.47879	0.47448	0.899%	0.46146	3.62%	0.45975	3.98%
2	0.49346	0.48139	2.44%	0.47184	4.38%	0.47058	4.64%
∞	1	1	0	1	0	1	0

Table 2. Cont.

After two iterations, when $\varepsilon_1 = 0$, the convergence region of V_b is $0 < \alpha \le 0.5$. When $\varepsilon_1 = 24(\pi - 2)/\pi^3$, the convergence region of V_b is $0 < \alpha \le 1.5$, whence the convergence region is expanded by 200% compared to the case with $\varepsilon_1 = 0$.

When $0 < \alpha \le 0.5$, ε_1 increases from 0 to 1, the series of solutions of vertical displacement converge to the exact solution with relative error to the exact solution is less than 1%, so that the effective region of ε_1 is identified to be $R_{\varepsilon_1} = [0, 1]$.

Thus, the optimal control parameters for the convergence region of the improved homotopy analysis method is identified to be $\varepsilon_1 = 24(\pi - 2)/\pi^3$.

3.3.3. Control Parameters $h_1 \in [-1, 0)$ for Convergence Rate

By IHAM, when $\varepsilon = 8/\pi^2$, h = -0.5, $\varepsilon_1 = 24(\pi - 2)/\pi^3$, $h_1 = -0.5$, the 10th-order approximation of V_b is

$$V_{b} = \frac{1}{3}\alpha - 3.7639 \times 10^{-2}\alpha^{3} + 7.5564 \times 10^{-3}\alpha^{5}$$

-1.6013 × 10⁻³\alpha^{7} + 2.9034 × 10^{-4}\alpha^{9} - 4.1074 × 10^{-5}\alpha^{11}
+4.3984 × 10^{-6}\alpha^{13} - 3.5126 × 10^{-7}\alpha^{15} + 2.0566 × 10^{-8}\alpha^{17}
-8.2692 × 10^{-10}\alpha^{19} + 1.7210 × 10^{-11}\alpha^{21} (3.39)

When the relative error $\Delta \leq$ 1%, the convergent regions of the 10th-order approximation by IHAM are

$$\alpha \in [0, 3.3], V_b \in [0, 0.63], 0 \le \delta_v \le 0.63L$$
 (3.40)

Table 3 compares the 10th-order numerical solutions with the exact solutions. The relative error in Table 3 is calculated by

$$\Delta = \frac{\left| V_b^{[n=10]} - V_b \right|}{V_b} \times 100\%$$
(3.41)

Table 3. 10th-order numerical solutions $V_b^{[n=10]}$ by IHAM with different values of h_1 ($\varepsilon_1 = 24(\pi - 2)/\pi^3$) compared with exact solutions V_b .

~	Exact Solution	h ₁	= 1	h ₁ =	-0.5	$h_1 = -0.3$	
u	V_b	V_b	Δ	V_b	Δ	V_b	Δ
0	0	0	0	0	0	0	0
0.2	0.066364	0.066365	0.000951%	0.066368	0.00522%	0.066385	0.0316%
0.4	0.13098	0.13098	0.00286%	0.13100	0.0183%	0.13113	0.120%
0.6	0.19235	0.19236	0.00418%	0.19241	0.0339%	0.19283	0.252%
0.8	0.24945	0.24946	0.00464%	0.24957	0.0480%	0.25047	0.410%
1.0	0.30172	0.30174	0.00486%	0.30190	0.0602%	0.30348	0.583%
1.2	0.34901	0.34902	0.00369%	0.34926	0.0713%	0.35168	0.765%
1.4	0.39147	0.39145	0.00312%	0.39178	0.0803%	0.39519	0.950%
1.6	0.42941	0.42933	0.0201%	0.42977	0.0829%	0.43429	1.14%
1.8	0.46326	0.46305	0.0457%	0.46361	0.0750%	0.46936	1.32%
2.0	0.49346	0.49313	0.0665%	0.49374	00572%	0.50082	1.49%
2.2	0.52042	0.52014	0.0539%	0.52061	0.0371%	0.52908	1.66%

~	Exact Solution	h ₁	= 1	h ₁ =	-0.5	$h_1 = -0.3$	
u	V_b	V_b	Δ	V_b	Δ	V_b	Δ
2.4	0.54455	0.54472	0.0319%	0.54471	0.0296%	0.55457	1.84%
2.6	0.56619	0.56745	0.222%	0.56646	0.0475%	0.57767	2.03%
2.8	0.58567	0.58846	0.476%	0.58610	0.0741%	0.59864	2.21%
3	0.60325	0.60709	0.636%	0.60340	0.0243%	0.61745	2.35%
3.2	0.61918	0.62601	1.10%	0.61870	0.0777%	0.63428	2.44%
3.4	0.63365	0.68393	7.94%	0.64405	1.64%	0.65542	3.44%
3.6	0.64684	_	_	0.76181	17.8%	0.72504	12.1%
3.8	0.65890	_	_	_	_	_	_
4.0	0.66996		_	_	_	_	_
∞	1	1	0	1	0	1	0

Table 3. Cont.

After n = 10 iterative calculations, when $h_1 = -1$, the convergence region of V_b is $0 < \alpha \le 3$. When $h_1 = -0.5$, the convergence region of V_b is $0 < \alpha \le 3.2$. When $h_1 = -0.3$, the convergence region of V_b is $0 < \alpha \le 1.4$. It is seen that when $h_1 = -0.5$, the convergence region is 128% larger than that with $h_1 = -0.3$, while it is 6.7% larger than that with $h_1 = -1$ (See Table 3).

Accordingly, the optimal control parameters h_1 of convergence rate for the 10th-order approximation solution to the vertical displacement is identified to be $h_1 = -0.5$.

3.3.4. 30th-Order Approximation Solution of Vertical Displacement

When $\varepsilon = 8/\pi^2$, h = -0.1, the 30th-order approximation of θ_b by IHAM is

$$\begin{aligned} \theta_{b} &= \frac{\alpha}{2} - 4.3804 \times 10^{-2} \alpha^{3} + 7.2872 \times 10^{-3} \alpha^{5} - 1.3299 \times 10^{-3} \alpha^{7} \\ &+ 2.2991 \times 10^{-4} \alpha^{9} - 3.5481 \times 10^{-5} \alpha^{11} + 4.7492 \times 10^{-6} \alpha^{13} \\ &- 5.4234 \times 10^{-7} \alpha^{15} + 5.2245 \times 10^{-8} \alpha^{17} - 4.2081 \times 10^{-9} \alpha^{19} \\ &+ 2.8138 \times 10^{-10} \alpha^{21} - 1.5540 \times 10^{-11} \alpha^{23} + 7.0725 \times 10^{-13} \alpha^{25} \\ &- 2.6561 \times 10^{-14} \alpha^{27} + 8.2720 \times 10^{-16} \alpha^{29} - 2.1528 \times 10^{-17} \alpha^{31} \\ &+ 4.7252 \times 10^{-19} \alpha^{33} - 8.8286 \times 10^{-21} \alpha^{35} + 1.4162 \times 10^{-22} \alpha^{37} \\ &- 1.9643 \times 10^{-24} \alpha^{39} + 2.3681 \times 10^{-26} \alpha^{41} - 2.4891 \times 10^{-28} \alpha^{43} \\ &+ 2.2824 \times 10^{-30} \alpha^{45} - 1.8213 \times 10^{-32} \alpha^{47} + 1.2569 \times 10^{-34} \alpha^{49} \\ &- 7.4143 \times 10^{-37} \alpha^{51} + 3.6656 \times 10^{-39} \alpha^{53} - 1.4683 \times 10^{-41} \alpha^{55} \\ &+ 4.4741 \times 10^{-44} \alpha^{57} - 9.1359 \times 10^{-47} \alpha^{59} + 9.2009 \times 10^{-50} \alpha^{61} \end{aligned}$$

When the relative error $\Delta \leq$ 1%, the convergent region of the 30th-order approximation by IHAM is

$$\alpha \in [0, 5.1], \ \Theta_b \in [0, 0.79], \ 0 \le \theta_b \le 70.6^\circ$$
(3.43)

When $\varepsilon_1 = 0$, $h_1 = -0.3$, the 30th-order approximation of V_b by HAM is

$$\begin{split} V_b &= \frac{1}{3}\alpha - 3.6198 \times 10^{-2}\alpha^3 + 6.5568 \times 10^{-3}\alpha^5 \\ &- 1.2305 \times 10^{-3}\alpha^7 + 2.1160 \times 10^{-4}\alpha^9 - 3.1714 \times 10^{-5}\alpha^{11} \\ &+ 4.0427 \times 10^{-6}\alpha^{13} - 4.3197 \times 10^{-7}\alpha^{15} + 3.8284 \times 10^{-8}\alpha^{17} \\ &- 2.7902 \times 10^{-9}\alpha^{19} + 1.6610 \times 10^{-10}\alpha^{21} - 8.0436 \times 10^{-12}\alpha^{23} \\ &+ 3.1678 \times 10^{-13}\alpha^{25} - 1.0188 \times 10^{-14}\alpha^{27} + 2.6964 \times 10^{-16}\alpha^{29} \\ &- 5.9333 \times 10^{-18}\alpha^{31} + 1.0971 \times 10^{-19}\alpha^{33} - 1.7218 \times 10^{-21}\alpha^{35} \\ &+ 2.3130 \times 10^{-23}\alpha^{37} - 2.6772 \times 10^{-25}\alpha^{39} + 2.6808 \times 10^{-27}\alpha^{41} \\ &- 2.3258 \times 10^{-29}\alpha^{43} + 1.7456 \times 10^{-31}\alpha^{45} - 1.1275 \times 10^{-33}\alpha^{47} \\ &+ 6.2040 \times 10^{-36}\alpha^{49} - 2.8576 \times 10^{-38}\alpha^{51} + 1.0708 \times 10^{-40}\alpha^{53} \\ &- 3.0973 \times 10^{-43}\alpha^{55} + 6.1662 \times 10^{-46}\alpha^{57} - 6.2456 \times 10^{-49}\alpha^{59} \end{split}$$

Table 4 compares the above numerical solution with the exact solution. The relative error in Table 4 is calculated by

$$\Delta = \frac{\left| V_b^{[n=30]} - V_b \right|}{V_b} \times 100\%$$
(3.45)

Table 4. 30th-order numerical solutions $V_b^{[n=30]}$ by IHAM ($\varepsilon = 8/\pi^2$, h = -0.1, $h_1 = -0.3$) compared with exact solutions V_b .

α	Exact Solution	ε ₁ (Η4	= 0 AM)	$\varepsilon_1 = 24(z)$ (IH)	$(\pi - 2)/\pi^3$ AM)
	V _b	V _b	Δ	V _b	Δ
0	0	0	0	0	0
0.3	0.098991	0.099038	0.0481%	0.099038	0.0482%
0.6	0.19235	0.19266	0.160%	0.19266	0.162%
0.9	0.27621	0.27697	0.275%	0.27699	0.282%
1.2	0.34901	0.35025	0.355%	0.35030	0.370%
1.5	0.41098	0.41262	0.400%	0.41271	0.422%
1.8	0.46326	0.46522	0.423%	0.46534	0.449%
2.1	0.50732	0.50953	0.437%	0.50966	0.462%
2.4	0.54455	0.54699	0.449%	0.54709	0.467%
2.7	0.57618	0.57883	0.459%	0.57888	0.467%
3.0	0.60325	0.60609	0.470%	0.60605	0.464%
3.3	0.62658	0.62959	0.479%	0.62945	0.457%
3.6	0.64684	0.65001	0.490%	0.64975	0.450%
3.9	0.66455	0.66794	0.510%	0.66746	0.438%
4.2	0.68014	0.68338	0.475%	0.68282	0.393%
4.5	0.69397	0.69274	0.177%	0.69571	0.251%
4.8	0.70629	0.68406	3.15%	0.70724	0.135%
5.1	0.71735	0.65758	8.33%	0.72349	0.856%
5.4	0.72731	0.76120	4.66%	0.73988	1.73%
5.7	0.73635	_	_	0.68583	6.86%
6.0	0.74457	—	_	0.44959	39.6%
~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~	1	1	0	1	0

When the relative error  $\Delta \leq$  1%, the convergent regions of the 30th-order approximation by HAM ( $\epsilon_1=$  0) are

$$\alpha \in [0, 4.6], V_b \in [0, 0.70], 0 \le \delta_v \le 0.70L$$
 (3.46)

When  $\varepsilon_1 = 24(\pi - 2)/\pi^3$ ,  $h_1 = -0.3$ , the 30th-order approximation of  $V_b$  by IHAM is

$$\begin{split} V_b &= \frac{1}{3}\alpha - 3.6198 \times 10^{-2}\alpha^3 + 6.6118 \times 10^{-3}\alpha^5 - 1.2700 \times 10^{-3}\alpha^7 \\ &+ 2.2683 \times 10^{-4}\alpha^9 - 3.5802 \times 10^{-5}\alpha^{11} + 4.8726 \times 10^{-6}\alpha^{13} \\ &- 5.6396 \times 10^{-7}\alpha^{15} + 5.5006 \times 10^{-8}\alpha^{17} - 4.4899 \times 10^{-9}\alpha^{19} \\ &+ 3.0512 \times 10^{-10}\alpha^{21} - 1.7202 \times 10^{-11}\alpha^{23} + 8.0374 \times 10^{-13}\alpha^{25} \\ &- 3.1168 \times 10^{-14}\alpha^{27} + 1.0073 \times 10^{-15}\alpha^{29} - 2.7301 \times 10^{-17}\alpha^{31} \\ &+ 6.2517 \times 10^{-19}\alpha^{33} - 1.2190 \times 10^{-20}\alpha^{35} + 2.0389 \times 10^{-22}\alpha^{37} \\ &- 2.9432 \times 10^{-24}\alpha^{39} + 3.6841 \times 10^{-26}\alpha^{41} - 4.0095 \times 10^{-28}\alpha^{43} \\ &+ 3.7952 \times 10^{-30}\alpha^{45} - 3.1164 \times 10^{-32}\alpha^{47} + 2.2056 \times 10^{-34}\alpha^{49} \\ &- 1.3297 \times 10^{-36}\alpha^{51} + 6.6933 \times 10^{-39}\alpha^{53} - 2.7186 \times 10^{-41}\alpha^{55} \\ &+ 8.3645 \times 10^{-44}\alpha^{57} - 1.7196 \times 10^{-46}\alpha^{59} + 1.7421 \times 10^{-49}\alpha^{61} \end{split}$$

When the relative error  $\Delta \le 1\%$ , the convergent regions of the 30th-order approximation by IHAM ( $\varepsilon_1 = 24(\pi - 2)/\pi^3$ ) are

$$\alpha \in [0, 5.1], V_b \in [0, 0.72], 0 \le \delta_v \le 0.72L$$
 (3.48)

From Table 4, the improved homotopy analysis method extends the convergence region from  $\alpha \in [0, 4.6]$  by the homotopy analysis method [32] to  $\alpha \in [0, 5.1]$ , which is an 11% increase.

From the above comparison, it can be seen that the expansion rate of the convergence region for vertical displacement by IHAM against that by HAM increases with the increasing iterations.

# 3.3.5. Homotopy-Páde Approximation

By application of the homotopy-Páde acceleration technique [27], the convergence region can be greatly enlarged. For example, when n = 30,  $\varepsilon_1 = 0$ ,  $h_1 = -0.3$ , the *Pade*[11, 11] homotopy-Páde approximation of  $V_b$  in Equation (3.44) by HAM can be written as

$$V_b = \frac{\alpha}{3} \frac{f(\alpha)}{g(\alpha)}$$
(3.49a)

$$f(\alpha) = 1 + 0.31708\alpha^2 + 6.1434 \times 10^{-2}\alpha^4 + 6.0404 \times 10^{-3}\alpha^6 + 4.1539 \times 10^{-4}\alpha^8 + 5.5063 \times 10^{-6}\alpha^{10}$$
(3.49b)

$$g(\alpha) = 1 + 0.42567\alpha^2 + 8.7988 \times 10^{-2}\alpha^4 + 1.0914 \times 10^{-2}\alpha^6 + 8.0640 \times 10^{-4}\alpha^8 + 2.8143 \times 10^{-6}\alpha^{10}$$
(3.49c)

When the relative error  $\Delta \leq 1\%$ , the convergent regions of the *Pade*[11, 11] homotopy-Páde approximation by HAM are

$$\alpha \in [0,5], \ V_b \in [0,0.71], \ 0 \le \delta_v \le 0.71L \tag{3.50}$$

When n = 30,  $\varepsilon_1 = 24(\pi - 2)/\pi^3$ ,  $h_1 = -0.3$ , the *Pade*[15, 15] homotopy-Páde approximation of  $V_b$  in Equation (3.47) by IHAM are

$$V_b = \frac{\alpha}{3} \frac{f(\alpha)}{g(\alpha)}$$
(3.51a)

$$f(\alpha) = 1 + 0.33026\alpha^{2} + 7.1207 \times 10^{-2}\alpha^{4} + 8.9035 \times 10^{-3}\alpha^{6} + 9.2792 \times 10^{-4}\alpha^{8} + 5.3226 \times 10^{-5}\alpha^{10} + 2.5321 \times 10^{-6}\alpha^{12} + 1.9082 \times 10^{-8}\alpha^{14}$$
(3.51b)

$$g(\alpha) = 1 + 0.43885\alpha^{2} + 9.9028 \times 10^{-2}\alpha^{4} + 1.4763 \times 10^{-2}\alpha^{6} + 1.5584 \times 10^{-3}\alpha^{8} + 1.1572 \times 10^{-4}\alpha^{10} + 5.5647 \times 10^{-6}\alpha^{12} + 1.3297 \times 10^{-7}\alpha^{14}$$
(3.51c)

When the relative error  $\Delta \le 1\%$ , the convergent regions of the *Pade*[15, 15] homotopy-Páde approximation by IHAM are

$$\alpha \in [0, 8.1], \ V_b \in [0, 0.79], \ 0 \le \delta_v \le 0.79L \tag{3.52}$$

Table 5 shows that numerical solutions  $V_b^{[P]}$  of homotopy-Páde approximation by IHAM compares with exact solutions  $V_b$ . The relative error in Table 5 is calculated by

$$\Delta = \frac{\left|V_b^{[P]} - V_b\right|}{V_b} \times 100\% \tag{3.53}$$

**Table 5.** Numerical solutions  $V_b^{[P]}$  of homotopy-Páde approximation by IHAM. (n = 30,  $\varepsilon = 8/\pi^2$ , h = -0.1,  $h_1 = -0.3$ ) compared with exact solutions  $V_b$ .

α	Exact Solution	ε ₁ (Η Pade	= 0 AM) [11,11]	ε ₁ = 24(7 (IHA Pade[	$\epsilon_1 = 24(\pi - 2)/\pi^3$ (IHAM) Pade[15,15]	
	V _b	$V_b$	Δ	$V_b$	Δ	
0	0	0	0	0	0	
0.5	0.16214	0.16234	0.120%	0.16234	0.121%	
1	0.30172	0.30265	0.307%	0.30267	0.315%	
1.5	0.41098	0.41262	0.400%	0.41271	0.422%	
2	0.49346	0.49559	0.433%	0.49572	0.459%	
2.5	0.55566	0.55817	0.452%	0.55826	0.468%	
3	0.60325	0.60610	0.471%	0.60605	0.463%	
3.5	0.64039	0.64360	0.501%	0.64325	0.446%	
4	0.66996	0.67379	0.571%	0.67272	0.412%	
4.5	0.69397	0.69899	0.723%	0.69645	0.358%	
5	0.71379	0.72096	1.00%	0.71587	0.292%	
5.5	0.73042	0.74102	1.45%	0.73214	0.235%	
6	0.74457	0.76007	2.08%	0.74616	0.213%	
6.5	0.75676	0.77877	2.91%	0.75867	0.252%	
7	0.76737	0.79754	3.93%	0.77024	0.374%	
7.5	0.77670	0.81664	5.14%	0.78132	0594%	
8	0.78498	0.83624	6.53%	0.79222	0.922%	
8.5	0.79239	0.85642	8.08%	0.80319	1.36%	
9	0.79906	0.87725	9.79%	0.81439	1.92%	
9.5	0.80510	0.89871	11.6%	0.82593	2.59%	
10	0.81061	0.92080	13.6%	0.83788	3.36%	
	1	1	0	1	0	

From Equations (3.50) and (3.52), the improved homotopy analysis method extends the convergence region from  $\alpha \in [0, 5]$  to  $\alpha \in [0, 8.1]$  for the homotopy-Páde approximate solution compared with the homotopy analysis method [32], which is a 62% increase.

Figure 2 compares the 30th-order iteration solution and the exact solution of the vertical displacement. From Figure 2, it can be observed that

the difference between the linear solution and the exact solution is remarkable;

- the 30th-order improved homotopy analysis solution for  $\varepsilon_1 = 24(\pi 2)/\pi^3$  is much closer to the exact solution than does the 30th-order homotopy analysis solution for  $\varepsilon_1 = 0$ ;
- the convergence range and rate have been greatly improved by the expression by homotopy-Páde approximation;
- the Páde approximation solution *Pade*[15, 15] by IHAM is more approximating to the exact solution than does the Páde approximation solution *Pade*[11, 11] by HAM.





#### 4. Horizontal Displacement of Cantilever Beam by IHAM

#### 4.1. Zero Order Deformation Equation

The nonlinear boundary value Equations (2.3c) and (2.4c) are then solved by IHAM to give explicit expression for  $U_b$ . To start with,  $X(\xi)$  is expressed by the power series of  $\xi$ , so that

$$X(\xi) = \sum_{k=1}^{+\infty} c_k \xi^k \tag{4.1}$$

where  $c_k$  is a constant coefficient. Linear Equation (2.7c) is specified as initial guess. Accordingly, we choose

$$X_0(\xi) = \xi \tag{4.2}$$

The equivalent nonlinear equation of Equation (2.3c) is

$$X' - 1 + \frac{\varepsilon_2}{2!}\theta^2 = 0 \tag{4.3}$$

subject to boundary condition of

$$X(0) = 0$$
 (4.4)

where  $\varepsilon_2$  is the correction coefficient of the convergent region, and  $\varepsilon_2 \in [0, 1]$ . When  $\varepsilon_2 = 0$ , Equation (4.3) becomes the linear Equation (2.5c), so that

$$X' - 1 = 0 \tag{4.5}$$

When  $\varepsilon_2 = 1$ , Equation (4.3) becomes

$$X' - 1 + \frac{1}{2!}\theta^2 = 0 \tag{4.6}$$

Approximation expression of the second-order Taylor expansion of cosine function can be written as

$$\cos\theta \approx 1 - \frac{1}{2!}\theta^2 \tag{4.7}$$

According to the original Equation (2.3c) and the equivalent Equation (4.3), we define a nonlinear operator as

$$N_{u}[\mu(\xi;q),\psi(\xi,q),q,\varepsilon_{2}] = \frac{d\mu(\xi;q)}{d\xi} - \cos[q \cdot \psi(\xi,q)] + \frac{\varepsilon_{2}}{2!} (1-q^{2}) [\psi(\xi,q)]^{2}$$

$$(4.8)$$

where  $q \in [0, 1]$  is an embedding parameter;  $\mu(\xi; q)$ ,  $\psi(\xi; q)$  are the functions dependent on  $\xi$  and q;  $\varepsilon_2$  is a control parameter for the convergent region, and  $\varepsilon_2 \in [0, 1]$ .

When q = 0, there is

$$N_{u}[\mu(\xi;0),\psi(\xi,0),0,\varepsilon_{2}] = \frac{d\mu(\xi;0)}{d\xi} - 1 + \frac{\varepsilon_{2}}{2!}[\psi(\xi,0)]^{2}$$
(4.9)

If q = 1, it yields

$$N_{u}[\mu(\xi;1),\psi(\xi,1),1,\varepsilon_{2}] = \frac{d\mu(\xi;1)}{d\xi} - \sin[\psi(\xi,1)]$$
(4.10)

Specifying

$$\mathcal{L}_{u}[\mu(\xi;q)] = \frac{\partial \mu(\xi;q)}{\partial \xi}$$
(4.11)

as an auxiliary linear operator, and denoting  $h_2 \in [-1, 0)$  as a nonzero auxiliary parameter (convergence-control parameter), a homotopy is constructed as

$$\mathcal{H}_{u}[\mu(\xi;q),q] := (1-q)L_{u}[\mu(\xi;q) - X_{0}(\xi)] - qh_{2}N_{u}[\mu(\xi;q),\psi(\xi,q),q,\varepsilon_{2}]$$
(4.12)

Accordingly, when q = 0, we have

$$\mathcal{H}_{u}[\mu(\xi;0),0] := \mathcal{L}_{u}[\mu(\xi;0) - X_{0}(\xi)]$$
(4.13)

If q = 1, it becomes

$$\mathcal{H}_{u}[\mu(\xi;1),1] := -h_{2} \mathcal{N}_{u}[\mu(\xi;1),1,\varepsilon_{2}]$$

$$= -h_{2} \left\{ \frac{d\mu(\xi;1)}{d\xi} - \cos[\psi(\xi,1)] \right\}$$

$$(4.14)$$

Thus, by enforcing

$$\mathcal{H}_{u}[\mu(\xi;q),q] = 0 \tag{4.15}$$

we have a family of equations

$$(1-q)L_{u}[\mu(\xi;q) - X_{0}(\xi)] = qh_{2}N_{u}[\mu(\xi;q),\psi(\xi,q),q,\varepsilon_{2}]$$
(4.16)

subject to boundary conditions

$$\mu(0;q) = 0 \tag{4.17}$$

When q = 0, we have

$$\mu(\xi;0) = X_0(\xi), \ \psi(\xi;0) = \theta_0(\xi) \tag{4.18}$$

When q = 1, Equations (4.16) and (4.17) are equivalent to the original Equations (2.3c) and (2.4c), provided that

$$\mu(\xi; 1) = X(\xi), \ \psi(\xi; 1) = \theta(\xi)$$
(4.19)

Thus, as *q* increases from 0 to 1,  $\mu(\xi; q)$  evolves from the known initial guess solution  $X_0(\xi)$  to the exact solution  $X(\xi)$  of Equations (2.3c) and (2.4c).

With reference to Equation (4.10), by expanding  $\mu(\xi; q)$  into Taylor series of the embedding parameter q, we have

$$\mu(\xi;q) = X_0(\xi) + \sum_{n=1}^{+\infty} X_n(\xi)q^n$$
(4.20a)

$$\psi(\xi;q) = \theta_0(\xi) + \sum_{n=1}^{+\infty} \theta_n(\xi)q^n$$
(4.20b)

where

$$X_n = \frac{1}{n!} \frac{\partial^n \mu(\xi;q)}{\partial q^n} \Big|_{q=0}, \ \theta_n = \frac{1}{n!} \frac{\partial^n \psi(\xi;q)}{\partial q^n} \Big|_{q=0}$$
(4.21)

It is worth noting that the zero order deformation Equation (4.16) contains auxiliary parameters  $\varepsilon_2$ ,  $h_2$ . And the auxiliary function  $H_2(\xi; q, \varepsilon_2)$  has the form of

$$H_2(\xi; q, \varepsilon_2) = \frac{\varepsilon_2}{2!} \left( 1 - q^2 \right) [\psi(\xi, q)]^2$$
(4.22)

Assuming that  $h_2$  and  $\varepsilon_2$  are properly chosen, so that Equation (4.19) is convergent at q = 1, thus, we have

$$X(\xi) = X_0(\xi) + \sum_{n=1}^{+\infty} X_n(\xi)$$
(4.23)

The governing equations  $X_n(\xi)$  can be deduced from 0th-order deformation Equations (4.16) and (4.17).

# 4.2. High Order Deformation Equations

Substituting Equation (4.20) into Equation (4.16) and differentiating (4.16) n times with respect to the embedding parameter q, then dividing by n! and setting q = 0, we have the n th-order deformation equation as below

$$L_{u}[X_{n}(\xi) - \chi_{n}X_{n-1}(\xi)] = h_{2}R_{n}^{u}(X_{0}, X_{1}, \cdots, X_{n-1}; \theta_{0}, \theta_{1}, \cdots, \theta_{n-1})$$
(4.24)

Equation (4.24) is subject to boundary condition of

$$X_n(0) = 0$$
 (4.25)

where

$$R_n^u = \left. \frac{1}{(n-1)!} \frac{\partial^{n-1} \mathcal{N}_u[\mu(\xi;q),\psi(\xi;q),q,\varepsilon_2]}{\partial q^{n-1}} \right|_{q=0}$$
(4.26)

and

$$\chi_n = \begin{cases} 0, n \le 1\\ 1, n > 1 \end{cases}$$
(4.27)

Combining Equations (4.8) and (4.26), we have

$$R_1^u = X'_0(\xi) - 1 + \frac{\varepsilon_2}{2}\theta_0^2(\xi)$$
(4.28a)

$$R_2^{u} = X'_1(\xi) + \varepsilon_2 \theta_0(\xi) \theta_1(\xi)$$
(4.28b)

$$R_3^{u} = X'_2(\xi) + \frac{1}{2}\theta_0^2(\xi) - \frac{\varepsilon_2}{2}\theta_0^2(\xi) + \frac{\varepsilon_2}{2}\theta_1^2(\xi) + \varepsilon_2\theta_0(\xi)\theta_2(\xi)$$
(4.28c)

$$R_4^u = X'_3(\xi) + \theta_0(\xi)\theta_1(\xi)$$
  

$$\epsilon_2\theta_0(\xi)\theta_1(\xi) + \epsilon_2\theta_1(\xi)\theta_2(\xi) + \epsilon_2\theta_0(\xi)\theta_3(\xi)$$
(4.28d)

$$R_{5}^{u} = X_{4}'(\xi) + \theta_{0}(\xi)\theta_{2}(\xi) + \frac{1}{2}\theta_{1}^{2}(\xi) - \frac{1}{24}\theta_{0}^{4}(\xi) - \varepsilon_{2}\theta_{0}(\xi)\theta_{2}(\xi) + \varepsilon_{2}\theta_{1}(\xi)\theta_{3}(\xi) + \varepsilon_{2}\theta_{0}(\xi)\theta_{4}(\xi) - \frac{\varepsilon_{2}}{2}\theta_{1}^{2}(\xi) + \frac{\varepsilon_{2}}{2}\theta_{2}^{2}(\xi)$$
(4.28e)

The right-hand side term  $R_n^u$  of the above equations can be derived by symbolic calculation software such as Maple, Matlab, etc. Thereafter, explicit form of solution to the linear high-order deformation Equations (4.24) and (4.25) have been derived.

#### 4.3. Horizontal Displacement at Free End of Cantilever Beam

# 4.3.1. Accurate Solution to Horizontal Displacement

Accordingly, in Equation (2.3c), the transcendental equation to solve  $U_b$  is as follows

$$U_b = 1 - \sqrt{\frac{2\sin\theta_b}{\alpha}} \tag{4.29}$$

With reference to Equation (2.8c), the linear solution of the horizontal displacement at the free end is

$$U_b = 0 \tag{4.30}$$

## 4.3.2. Explicit Solution to Horizontal Displacement by IHAM

The solutions by IHAM contains two auxiliary parameters  $\varepsilon_2$  and  $h_2$ , which control and adjust the convergence region and rate of IHAM solution series.

For example, the 10th-order approximation of  $U_b$  with n = 10,  $\varepsilon = 8/\pi^2$ , h = -0.5,  $\varepsilon_2 = 8/\pi^2$ ,  $h_2 = -0.4$  is derived as

$$\begin{aligned} U_b &= 6.6128 \times 10^{-2} \alpha^2 - 1.2132 \times 10^{-2} \alpha^4 + 2.6029 \times 10^{-3} \alpha^6 \\ &- 5.0108 \times 10^{-4} \alpha^8 + 7.4259 \times 10^{-5} \alpha^{10} - 8.1166 \times 10^{-6} \alpha^{12} \\ &+ 6.5090 \times 10^{-7} \alpha^{14} - 3.7900 \times 10^{-8} \alpha^{16} + 1.5743 \times 10^{-9} \alpha^{18} \\ &- 3.4811 \times 10^{-11} \alpha^{20} \end{aligned}$$
(4.31)

When the relative error  $\Delta \leq$  1%, the convergent regions of the 10th-order approximation by IHAM are

$$\alpha \in [0, 2.8], \ U_b \in [0, 0.24], \ 0 \le \delta_h \le 0.24L$$
 (4.32)

The 20th-order approximation of  $U_b$  with n = 20,  $\varepsilon = 8/\pi^2$ , h = -0.3,  $\varepsilon_2 = 8/\pi^2$ ,  $h_2 = -0.4$  is expressed as

$$\begin{split} & U_b = 6.6663 \times 10^{-2} \alpha^2 - 1.2647 \times 10^{-2} \alpha^4 + 3.0553 \times 10^{-3} \alpha^6 \\ & -7.8913 \times 10^{-4} \alpha^8 + 1.9565 \times 10^{-4} \alpha^{10} - 4.2939 \times 10^{-5} \alpha^{12} \\ & +7.8795 \times 10^{-6} \alpha^{14} - 1.1689 \times 10^{-6} \alpha^{16} + 1.3828 \times 10^{-7} \alpha^{18} \\ & -1.3058 \times 10^{-8} \alpha^{20} + 9.9286 \times 10^{-10} \alpha^{22} - 6.1404 \times 10^{-11} \alpha^{24} \\ & +3.1142 \times 10^{-12} \alpha^{26} - 1.3002 \times 10^{-13} \alpha^{28} + 4.4587 \times 10^{-15} \alpha^{30} \\ & -1.2419 \times 10^{-16} \alpha^{32} + 2.7429 \times 10^{-18} \alpha^{34} - 4.5778 \times 10^{-20} \alpha^{36} \\ & +5.1643 \times 10^{-22} \alpha^{38} - 2.9055 \times 10^{-24} \alpha^{40} \end{split}$$

When the relative error  $\Delta \leq 1\%$ , the convergent region of the 20th-order approximation by IHAM is

$$\alpha \in [0, 3.4], \ U_b \in [0, 0.29], \ 0 \le \delta_h \le 0.29L$$
 (4.34)

The 30th-order approximation of  $U_b$  with n = 30,  $\varepsilon = 8/\pi^2$ , h = -0.1,  $\varepsilon_2 = 8/\pi^2$ ,  $h_2 = -0.3$  is

$$\begin{split} & U_b = 6.6665 \times 10^{-2} \alpha^2 - 1.1891 \times 10^{-2} \alpha^4 + 2.4037 \times 10^{-3} \alpha^6 \\ & -4.6144 \times 10^{-4} \alpha^8 + 7.8466 \times 10^{-5} \alpha^{10} - 1.1452 \times 10^{-5} \alpha^{12} \\ & +1.4110 \times 10^{-6} \alpha^{14} - 1.4525 \times 10^{-7} \alpha^{16} + 1.2395 \times 10^{-8} \alpha^{18} \\ & -8.7140 \times 10^{-10} \alpha^{20} + 5.0250 \times 10^{-11} \alpha^{22} - 2.3731 \times 10^{-12} \alpha^{24} \\ & +9.1975 \times 10^{-14} \alpha^{26} - 2.9421 \times 10^{-15} \alpha^{28} + 7.8331 \times 10^{-17} \alpha^{30} \\ & -1.7531 \times 10^{-18} \alpha^{32} + 3.3309 \times 10^{-20} \alpha^{34} - 5.4217 \times 10^{-22} \alpha^{36} \\ & +7.6171 \times 10^{-24} \alpha^{38} - 9.2885 \times 10^{-26} \alpha^{40} + 9.8639 \times 10^{-28} \alpha^{42} \\ & -9.1295 \times 10^{-30} \alpha^{44} + 7.3481 \times 10^{-32} \alpha^{46} - 5.1117 \times 10^{-34} \alpha^{48} \\ & +3.0383 \times 10^{-36} \alpha^{50} - 1.5131 \times 10^{-38} \alpha^{52} + 6.1056 \times 10^{-41} \alpha^{54} \\ & -1.8743 \times 10^{-43} \alpha^{56} + 3.8564 \times 10^{-46} \alpha^{58} - 3.9126 \times 10^{-49} \alpha^{60} \end{split}$$

When the relative error  $\Delta \leq$  1%, the convergent regions of the 30th-order approximation by IHAM are

$$\alpha \in [0, 4.6], \ U_b \in [0, 0.37], \ 0 \le \delta_h \le 0.37L \tag{4.36}$$

Table 6 compares the above numerical solutions with the exact solutions. The relative error in Table 6 is calculated by

$$\Delta = \frac{\left| U_b^{[n=30]} - U_b \right|}{U_b} \times 100\% \tag{4.37}$$

**Table 6.** 30th-order numerical solutions  $U_b^{[n=30]}$  by IHAM ( $\varepsilon = 8/\pi^2$ , h = -0.1,  $h_2 = -0.3$ ) compared with exact solutions  $U_b$ .

α	Exact Solution	ε ₂ : (HA	= 0 M)	$\varepsilon_2 = (IHA)$	8/π ² AM)
	U _b	U _b	Δ	U _b	Δ
0	0	0	0	0	0
0.3	0.0058994	0.0059063	0.116%	0.0059052	0.0993%
0.6	0.022488	0.022578	0.402%	0.022563	0.335%
0.9	0.047034	0.047359	0.690%	0.047300	0.565%
1.2	0.076400	0.077065	0.870%	0.076933	0.696%
1.5	0.10794	0.10894	0.923%	0.10872	0.723%
1.8	0.13981	0.14104	0.886%	0.14076	0.681%
2.1	0.17085	0.17223	0.804%	0.17190	0.610%
2.4	0.20046	0.20188	0.705%	0.20153	0.531%
2.7	0.22835	0.22973	0.603%	0.22939	0.456%
3.0	0.25442	0.25570	0.504%	0.25540	0.387%
3.3	0.27870	0.27985	0.411%	0.27961	0.324%
3.6	0.30128	0.30226	0.324%	0.30210	0.270%
3.9	0.32228	0.32316	0.274%	0.32310	0.257%
4.2	0.34180	0.34334	0.448%	0.34302	0.356%
4.5	0.35999	0.36371	1.03%	0.36042	0.121%
4.8	0.37695	0.37525	0.449%	0.36186	4.00%

 $\infty$ 

α	Exact Solution	ε ₂ : (HA	= 0 M)	$\varepsilon_2 = (IHA)$	8/π ² AM)
	Ub	U _b	Δ	U _b	Δ
5.1	0.39279	0.31920	18.7%	0.30447	22.5%
5.4	0.40762	0.027011	93.4%	0.14924	63.4%
5.7	0.42153	_	_	0.10903	74.1%
6.0	0.43459	_	_		

1

Table 6. Cont.

1

From Table 6, IHAM extends the convergence regions from  $\alpha \in [0, 4.2]$  by HAM to  $\alpha \in [0, 4.6]$ , which is 7.1% increase.

1

Figure 3 shows the relationship between horizontal displacement and concentrated force at the free end. From Figure 3, it is observed that

• the difference between the linear solution and the exact solution is remarkable;

0

• from n = 10 result of Equation (4.32), n = 20 result of Equation (4.34) and n = 30 result of Equation (4.37), the convergence regions of horizontal displacement increases with increasing iterations.



Figure 3. Horizontal displacement at free end.

#### 5. Discussions and Conclusions

5.1. Error Analysis of Vertical Displacement

The influence of the control parameters  $\varepsilon_1$  and  $h_1$  on the error of vertical displacement after n = 5 iterations with  $\alpha = 2.0$  is then discussed.

After five iterations, the improved homotopy analysis method is adopted to solve the Equations (3.21) and (3.22) to yield the solution to the original Equations (2.3b) and (2.4b) as below

0

$$\begin{split} V_{b} &= 0.33333\alpha + 0.096738h_{1}\alpha^{3} + 0.10197h_{1}^{2}\alpha^{3} \\ &+ 0.052108h_{1}^{3}\alpha^{3} + 0.011580h_{1}^{4}\alpha^{3} + 0.028571\varepsilon_{1}h_{1}\alpha^{3} \\ &+ 0.085714\varepsilon_{1}h_{1}^{2}\alpha^{3} + 0.095238\varepsilon_{1}h_{1}^{3}\alpha^{3} + 0.047619\varepsilon_{1}h_{1}^{4}\alpha^{3} \\ &+ 9.5238 \times 10^{-3}\varepsilon_{1}h_{1}^{5}\alpha^{3} - 6.1695 \times 10^{-3}h_{1}\alpha^{5} - 3.3120 \times 10^{-3}h_{1}^{2}\alpha^{5} \\ &- 8.2799 \times 10^{-4}h_{1}^{3}\alpha^{5} - 5.8320 \times 10^{-3}\varepsilon_{1}h_{1}\alpha^{5} - 8.1553 \times 10^{-3}\varepsilon_{1}h_{1}^{2}\alpha^{5} \\ &- 4.5967 \times 10^{-3}\varepsilon_{1}h_{1}^{3}\alpha^{5} - 1.0215 \times 10^{-3}\varepsilon_{1}h_{1}^{4}\alpha^{5} + 2.5955 \times 10^{-4}h_{1}\alpha^{7} \\ &+ 7.4156 \times 10^{-5}h_{1}^{2}\alpha^{7} + 6.8257 \times 10^{-4}\varepsilon_{1}h_{1}\alpha^{7} + 4.3913 \times 10^{-4}\varepsilon_{1}h_{1}^{2}\alpha^{7} \\ &+ 1.0978 \times 10^{-4}\varepsilon_{1}h_{1}^{3}\alpha^{7} - 7.4415 \times 10^{-6}h_{1}\alpha^{9} - 4.2843 \times 10^{-5}\varepsilon_{1}h_{1}\alpha^{9} \\ &- 1.2241 \times 10^{-5}\varepsilon_{1}h_{1}^{2}\alpha^{9} + 1.4103 \times 10^{-6}\varepsilon_{1}h_{1}\alpha^{11} \end{split}$$

When  $\alpha$  = 2.0, Equation (5.1) becomes

$$V_{b} = 0.66667 + 0.60589h_{1} + 0.71928h_{1}^{2} + 0.39037h_{1}^{3} + 0.092637h_{1}^{4} + 0.11027\varepsilon_{1}h_{1} + 0.47469\varepsilon_{1}h_{1}^{2} + 0.62886\varepsilon_{1}h_{1}^{3} + 0.34826\varepsilon_{1}h_{1}^{4} + 0.076190\varepsilon_{1}h_{1}^{5}$$
(5.2)

When  $\alpha$  = 2.0, the exact solution is  $V_b$  = 0.49346. When the iteration is five times, the relative error between the result by IHAM and the exact solution can be expressed as

$$\Delta_V = \frac{\left| V_b^{[n=5]} - V_b \right|}{V_b} \times 100\%$$
(5.3)

When  $h_1 = -0.68$ , Equation (5.2) becomes

$$V_b = 0.48432 + 0.010163\varepsilon_1 \tag{5.4}$$

According to Equations (5.3) and (5.4), the relative error curve of control parameter  $\Delta_V - \varepsilon_1$  is a broken line. When the relative error is less than  $\Delta_V \leq 1\%$ , the convergence interval is  $R_{\varepsilon_1} = [0.4, 1]$ , which is the effective region of  $\varepsilon_1$ .

Figure 4 shows the relative errors curve  $\Delta_V - \varepsilon_1$  of Equation (2.3b) when  $\alpha = 2.0$ ,  $h_1 = -0.68$ . From Figure 4, when  $\varepsilon_1 = 24(\pi - 2)/\pi^3$ , the relative error is the minimum.



**Figure 4.** Relative errors curve  $\Delta_V - \varepsilon_1$  of Equation (2.3b) when  $\alpha = 2.0$ ,  $h_1 = -0.68$ .

When  $\varepsilon_1 = 24(\pi - 2) / \pi^3$ , Equation (5.2) becomes

$$V_b = 0.66667 + 0.70333h_1 + 1.1387h_1^2 + 0.94605h_1^3 + 0.40038h_1^4 + 0.067325h_1^5$$
(5.5)

According to Equations (5.3) and (5.5), the relative error curve of control parameter  $\Delta_V - h_1$  is a quintic power function. When the relative error is less than  $\Delta_V \leq 1\%$ , the convergence interval is  $R_{h_1} = [-1, -0.58]$ , which is the effective region of  $h_1$ .

Figure 5 gives the relative errors curve  $\Delta_V - h_1$  of Equation (2.3b) when  $\alpha = 2.0$ ,  $\varepsilon_1 = 24(\pi - 2)/\pi^3$ . From Figure 5, when  $h_1 = -0.68$ , the relative error is the minimum.



**Figure 5.** Relative errors curve  $\Delta_V - h_1$  of Equation (2.3b) when  $\alpha = 2.0$ ,  $\varepsilon_1 = 24(\pi - 2)/\pi^3$ .

# 5.2. Error Analysis of Horizontal Displacement

The influence of the control parameter  $\varepsilon_2$  and  $h_2$  on the error of horizontal displacement after 5 iterations when  $\alpha = 2.0$  is then discussed.

After five iterations, the improved homotopy analysis method is adopted to solve the Equations (4.24) and (4.25) to derive the solution to the original Equations (2.3c) and (2.4c). Thus,

$$\begin{aligned} U_{b} &= -0.2000h_{2}\alpha^{2} - 0.2000h_{2}^{2}\alpha^{2} - 0.066667h_{2}^{3}\alpha^{2} \\ &- 0.13333\varepsilon_{2}h_{2}\alpha^{2} - 0.46667\varepsilon_{2}h_{2}^{2}\alpha^{2} - 0.60000\varepsilon_{2}h_{2}^{3}\alpha^{2} \\ &- 0.33333\varepsilon_{2}h_{2}^{4}\alpha^{2} - 0.066667\varepsilon_{2}h_{2}^{5}\alpha^{2} + 0.010871h_{2}\alpha^{4} \\ &+ 3.7741 \times 10^{-3}h_{2}^{2}\alpha^{4} + 0.017903\varepsilon_{2}h_{2}\alpha^{4} + 0.027904\varepsilon_{2}h_{2}^{2}\alpha^{4} \\ &+ 0.017361\varepsilon_{2}h_{2}^{3}\alpha^{4} + 3.7741 \times 10^{-3}\varepsilon_{2}h_{2}^{4}\alpha^{4} - 2.6973 \times 10^{-4}h_{2}\alpha^{6} \\ &- 1.6042 \times 10^{-3}\varepsilon_{2}h_{2}\alpha^{6} - 1.1329 \times 10^{-3}\varepsilon_{2}h_{2}^{2}\alpha^{6} \\ &- 2.6973 \times 10^{-4}\varepsilon_{2}h_{2}^{3}\alpha^{6} + 8.2194 \times 10^{-5}\varepsilon_{2}h_{2}\alpha^{8} \\ &+ 2.1630 \times 10^{-5}\varepsilon_{2}h_{2}^{2}\alpha^{8} - 1.8596 \times 10^{-6}\varepsilon_{2}h_{2}\alpha^{10} \end{aligned}$$
(5.6)

When  $\alpha$  = 2.0, Equation (5.6) becomes

$$U_{b} = -0.64333h_{2} - 0.73961h_{2}^{2} - 0.26667h_{2}^{3}$$
  
-0.33042\varepsilon_{2}h_{2} - 1.4872\varepsilon_{2}h_{2}^{2} - 2.1395\varepsilon_{2}h_{2}^{3}  
-1.2729\varepsilon_{2}h_{2}^{4} - 0.26667\varepsilon_{2}h_{2}^{5}  
(5.7)

When  $\alpha$  = 2.0, the exact solution is  $U_b$  = 0.16064. After 5 time iterations, the relative error between the results by IHAM method and the exact solution can be written as

$$\Delta_U = \frac{\left| U_b^{[n=5]} - U_b \right|}{U_b} \times 100\% \tag{5.8}$$

When  $h_2 = -0.465$ , Equation (5.7) becomes

$$U_b = 0.16603 - 0.0065196\varepsilon_2 \tag{5.9}$$

According to Equations (5.8) and (5.9), the relative error curve of control parameter  $\Delta_U - \varepsilon_2$  is a broken line. When the relative error is less than  $\Delta_U \leq 1\%$ , the convergence region is  $R_{\varepsilon_2} = [0.58, 1]$ , which is the effective region of  $\varepsilon_2$ .

Figure 6 shows the relative errors curve  $\Delta_U - \varepsilon_2$  of Equation (2.3c) when  $\alpha = 2.0$ ,  $h_2 = -0.465$ . From Figure 6, when  $\varepsilon_2 = 8/\pi^2$ , the relative error is the minimum.



**Figure 6.** Relative errors curve  $\Delta_U - \varepsilon_2$  of Equation (2.3c) when  $\alpha = 2.0$ ,  $h_2 = -0.465$ .

When  $\varepsilon_2 = 8/\pi^2$ , Equation (5.7) becomes

$$U_b = -0.91116h_2 - 1.9451h_2^2 - 2.0009h_2^3 - 1.0318h_2^4 - 0.21615h_2^5$$
(5.10)

According to Equations (5.8) and (5.10), the relative error curve of control parameter  $\Delta_U - h_2$  is a quintic polynomial curve. When the relative error is less than  $\Delta_U \leq 1\%$ , the convergence interval is  $R_{h_2} = [-0.75, -0.42]$ , which is the effective region of  $h_2$ .

Figure 7 depicts the relative errors curve  $\Delta_U - h_2$  of Equation (2.3c) when  $\alpha = 2.0$ ,  $\varepsilon_2 = 8/\pi^2$ . From Figure 7, when  $h_2 = -0.465$  or  $h_2 = -0.66$ , the relative error is the minimum.



**Figure 7.** Relative errors curve  $\Delta_U - h_2$  of Equation (2.3c) when  $\alpha = 2.0$ ,  $\varepsilon_2 = 8/\pi^2$ .

# 5.3. Conclusions

In this paper, an improved homotopy analysis method (IHAM) is applied to derive complete expressions of vertical and horizontal displacements of a cantilever beam under point load at the free end. In fact, the proposed method is a semi-analytical method since many numerical algorithms are used. The linear solution is only valid for small  $\alpha$ . For large  $\alpha$ , the nonlinear effect of the equation cannot be neglected. A family of the explicit solutions of the rotation angle, the vertical and horizontal displacements of a cantilever beam under a point load at the free end are obtained by IHAM, which are summarized in Table A1 for 30th-order approximation. The current explicit solutions with different parameters and solution schemes match well with the exact solutions from elliptical integrals; however, our solutions for the current problem by the proposed IHAM, especially Equations (3.36) and (4.31), are explicit and simple compared with the exact solution from the elliptical integrals which requires solving a transcendental equation. The current solution is easy to calculate with the explicit polynomial expressions, which should be, thus, prioritized for practical engineering applications with minimal requirements on the calculation and computation.

By solving the exact bending equation of a cantilever beam, we demonstrate the superior proficiency of IHAM over a class of existing equations in solid mechanics. The improved homotopy analysis method has great superiority compared over the traditional homotopy analysis method in view of the very much increased rate and precision of convergence as well as the substantially enlarged convergence range.

The convergence ranges of the rotation angle and vertical displacement are extended by the homotopy-Páde approximation, whereas it does not avail horizontal displacement.

In summary, the 30th-order improved homotopy analysis solutions and exact solutions of the angular, vertical and horizontal displacements of large deflection cantilever beams by Equations (3.42), (3.47) and (4.35) are compared in the following Appendix A table. It can be concluded from this study that the proposed IHAM can be applied to solve a general strong nonlinear problem in structural analysis.

**Author Contributions:** Conceptualization and methodogoly, Y.L. and X.L.; validation and data curation, C.X. and S.H.; formal analysis, Y.L.; investigation, X.L.; writing—original draft preparation, C.X.; writing—review and editing, X.L.; visualization, S.H.; supervision, Y.L.; project administration, Y.L.; funding acquisition, X.L. All authors have read and agreed to the published version of the manuscript.

**Funding:** This research was supported by the National Natural Science Foundation of China (Grant No. 11972145) and Tianjin Excellent Science and Technology Commissioner Support Project (Grant No. 20YDTPJC0080).

Institutional Review Board Statement: No application.

Informed Consent Statement: No application.

Data Availability Statement: No application.

Acknowledgments: This work was funded by the National Natural Science Foundation of China (Grant No. 11972145) and Tianjin Excellent Science and Technology Commissioner Support Project (Grant No. 20YDTPJC0080).

Conflicts of Interest: The authors declare no conflict of interest.

# Appendix A

**Table A1.** 30th-order numerical solutions  $\Theta_b^{[n=30]}$ ,  $V_b^{[n=30]}$ ,  $U_b^{[n=30]}$  by IHAM compared with exact solutions  $\Theta_b$ ,  $V_b$  and  $U_b$ .

	Rotation Angle $\Theta_b$		Vertical Dis	splacement V _b	Horizontal Displacement $U_b$	
α	Exact Solutions	n = 30 $\varepsilon = 8/\pi^2$ h = -0.1	Exact Solutions	$\begin{array}{c} n=30 \\ \epsilon_1=24(\pi\!-\!2)/\pi^3 \\ h_1=-0.3 \end{array}$	Exact Solutions	n = 30 $\epsilon_2 = 8/\pi^2$ $h_2 = -0.3$
0	0	0	0		0	0
0.3	0.094719	0.094751	0.098991	0.099038	0.0058994	0.0059052
0.6	0.18509	0.18530	0.19235	0.19266	0.022488	0.022563
0.9	0.26801	0.26853	0.27621	0.27699	0.047034	0.047300

$n$ Exact Solutions $n = 30$ $r = 8/r_1^2$ h = -0.1Exact Solutions $n = 30$ $r_1 = 24(\pi - 2)/r^3$ Exact Solutions $n = 30$ $r_2 = 8/r^2$ h_2 = -0.31.20.342060.342910.349010.350300.0764000.0769331.50.407140.408250.410980.412710.107940.108721.80.463940.465200.463260.465340.139810.140762.10.513420.514770.507320.509660.170850.21132.40.556610.557980.544550.547090.200460.21532.70.594440.595800.576180.578880.228350.229393.00.627720.629050.603250.606050.254420.255403.30.657140.658430.626580.629450.301280.302103.40.76590.707740.664550.667460.322280.323104.20.727490.728210.680140.682820.341800.343024.50.766300.746640.699770.695710.359990.360424.80.76329-0.707240.37695-5.10.77870-0.72731-0.40762-5.40.79272-0.72731-0.42153-5.70.80552-0.73635-0.42153-6.00.81723-0.74575-0.42153-		Rotation Angle $\Theta_b$		Vertical Dis	splacement V _b	Horizontal Dis	placement U _b
1.2 $0.34206$ $0.34291$ $0.34901$ $0.35030$ $0.076400$ $0.076933$ $1.5$ $0.40714$ $0.40825$ $0.41098$ $0.41271$ $0.10794$ $0.10872$ $1.8$ $0.46394$ $0.46520$ $0.46326$ $0.46534$ $0.13981$ $0.14076$ $2.1$ $0.51342$ $0.51477$ $0.50732$ $0.50966$ $0.17085$ $0.17190$ $2.4$ $0.55661$ $0.55798$ $0.54455$ $0.54709$ $0.20046$ $0.20153$ $2.7$ $0.59444$ $0.59580$ $0.57618$ $0.57888$ $0.22835$ $0.22939$ $3.0$ $0.62772$ $0.62905$ $0.60325$ $0.66005$ $0.25442$ $0.25540$ $3.3$ $0.65714$ $0.65843$ $0.62658$ $0.62945$ $0.27870$ $0.27961$ $3.6$ $0.68327$ $0.68452$ $0.66455$ $0.66746$ $0.32228$ $0.3210$ $4.2$ $0.72749$ $0.72821$ $0.68014$ $0.68282$ $0.34180$ $0.34302$ $4.5$ $0.74630$ $0.74664$ $0.69397$ $0.69571$ $0.35999$ $0.36042$ $4.8$ $0.76329$ $ 0.77231$ $ 0.40762$ $ 5.7$ $0.80552$ $ 0.73635$ $ 0.42153$ $ 6.0$ $0.81723$ $ 0.74457$ $ 0.42153$ $-$	α	Exact Solutions	n = 30 $\varepsilon = 8/\pi^2$ h = -0.1	Exact Solutions	$\begin{array}{c} n=30 \\ \epsilon_1=24(\pi\!-\!2)/\pi^3 \\ h_1=-0.3 \end{array}$	Exact Solutions	n = 30 $\epsilon_2 = 8/\pi^2$ $h_2 = -0.3$
1.5 $0.40714$ $0.40825$ $0.41098$ $0.41271$ $0.10794$ $0.10872$ $1.8$ $0.46394$ $0.46520$ $0.46326$ $0.46534$ $0.13981$ $0.14076$ $2.1$ $0.51342$ $0.51477$ $0.50732$ $0.50966$ $0.17085$ $0.17190$ $2.4$ $0.55661$ $0.55798$ $0.54455$ $0.54709$ $0.20046$ $0.20153$ $2.7$ $0.59444$ $0.59580$ $0.57618$ $0.57888$ $0.22835$ $0.22939$ $3.0$ $0.62772$ $0.62905$ $0.60325$ $0.66005$ $0.25442$ $0.25540$ $3.3$ $0.65714$ $0.65843$ $0.62658$ $0.62945$ $0.27870$ $0.27961$ $3.6$ $0.68327$ $0.68452$ $0.64684$ $0.64975$ $0.30128$ $0.30210$ $3.9$ $0.70659$ $0.70774$ $0.66455$ $0.66746$ $0.32228$ $0.32310$ $4.2$ $0.72749$ $0.72821$ $0.68014$ $0.68282$ $0.34180$ $0.34302$ $4.5$ $0.74630$ $0.74664$ $0.69397$ $0.69571$ $0.35999$ $0.36042$ $4.8$ $0.76329$ $ 0.70724$ $0.37695$ $ 5.1$ $0.77870$ $ 0.72731$ $ 0.40762$ $ 5.7$ $0.80552$ $ 0.73635$ $ 0.42153$ $ 6.0$ $0.81723$ $ 0.74457$ $ 0.43459$ $ 6.0$ $0.81723$ $ 0.74457$ $ 0.43459$ $-$	1.2	0.34206	0.34291	0.34901	0.35030	0.076400	0.076933
1.8 $0.46394$ $0.46520$ $0.46326$ $0.46534$ $0.13981$ $0.14076$ $2.1$ $0.51342$ $0.51477$ $0.50732$ $0.50966$ $0.17085$ $0.17190$ $2.4$ $0.55661$ $0.55798$ $0.54455$ $0.54709$ $0.20046$ $0.20153$ $2.7$ $0.59444$ $0.59580$ $0.57618$ $0.57888$ $0.22835$ $0.22939$ $3.0$ $0.62772$ $0.62905$ $0.60325$ $0.60605$ $0.25442$ $0.25540$ $3.3$ $0.65714$ $0.65843$ $0.62658$ $0.62945$ $0.27870$ $0.27961$ $3.6$ $0.68327$ $0.68452$ $0.64684$ $0.64975$ $0.30128$ $0.30210$ $3.9$ $0.70659$ $0.70774$ $0.66455$ $0.66746$ $0.32228$ $0.32310$ $4.2$ $0.72749$ $0.72821$ $0.68014$ $0.68282$ $0.34180$ $0.34302$ $4.5$ $0.74630$ $0.74664$ $0.69397$ $0.69571$ $0.35999$ $0.36042$ $4.8$ $0.76329$ $ 0.70735$ $0.72349$ $0.39279$ $ 5.1$ $0.77870$ $ 0.72731$ $ 0.40762$ $ 5.7$ $0.80552$ $ 0.73635$ $ 0.42153$ $ 6.0$ $0.81723$ $ 0.74457$ $ 0.43459$ $-$	1.5	0.40714	0.40825	0.41098	0.41271	0.10794	0.10872
2.1 $0.51342$ $0.51477$ $0.50732$ $0.50966$ $0.17085$ $0.17190$ $2.4$ $0.55661$ $0.55798$ $0.54455$ $0.54709$ $0.2046$ $0.20153$ $2.7$ $0.59444$ $0.59580$ $0.57618$ $0.57888$ $0.22835$ $0.22939$ $3.0$ $0.62772$ $0.62905$ $0.60325$ $0.60605$ $0.25442$ $0.25540$ $3.3$ $0.65714$ $0.65843$ $0.62658$ $0.62945$ $0.27870$ $0.27961$ $3.6$ $0.68327$ $0.68452$ $0.64684$ $0.64975$ $0.30128$ $0.30210$ $3.9$ $0.70659$ $0.70774$ $0.66455$ $0.66746$ $0.32228$ $0.32310$ $4.2$ $0.72749$ $0.72821$ $0.68014$ $0.69371$ $0.35999$ $0.36042$ $4.8$ $0.76329$ $ 0.70629$ $0.70724$ $0.37695$ $ 5.1$ $0.77870$ $ 0.71735$ $0.72349$ $0.39279$ $ 5.4$ $0.79272$ $ 0.72635$ $ 0.42153$ $ 6.0$ $0.81723$ $ 0.74457$ $ 0.43459$ $-$	1.8	0.46394	0.46520	0.46326	0.46534	0.13981	0.14076
2.4 $0.55661$ $0.55798$ $0.54455$ $0.54709$ $0.2046$ $0.20153$ $2.7$ $0.59444$ $0.59580$ $0.57618$ $0.57888$ $0.22835$ $0.22939$ $3.0$ $0.62772$ $0.62905$ $0.60325$ $0.60605$ $0.25442$ $0.25540$ $3.3$ $0.65714$ $0.65843$ $0.62658$ $0.62945$ $0.27870$ $0.27961$ $3.6$ $0.68327$ $0.68452$ $0.64684$ $0.64975$ $0.30128$ $0.30210$ $3.9$ $0.70659$ $0.70774$ $0.66455$ $0.66746$ $0.32228$ $0.32310$ $4.2$ $0.72749$ $0.72821$ $0.68014$ $0.68282$ $0.34180$ $0.34302$ $4.5$ $0.74630$ $0.74664$ $0.69397$ $0.69571$ $0.35999$ $0.36042$ $4.8$ $0.76329$ $ 0.70629$ $0.70724$ $0.37695$ $ 5.1$ $0.77870$ $ 0.71735$ $0.72349$ $0.39279$ $ 5.4$ $0.79272$ $ 0.72635$ $ 0.42153$ $ 5.7$ $0.80552$ $ 0.73635$ $ 0.42153$ $ 6.0$ $0.81723$ $ 0.74457$ $ 0.43459$ $-$	2.1	0.51342	0.51477	0.50732	0.50966	0.17085	0.17190
2.7 $0.59444$ $0.59580$ $0.57618$ $0.57888$ $0.22835$ $0.22939$ $3.0$ $0.62772$ $0.62905$ $0.60325$ $0.60605$ $0.25442$ $0.25540$ $3.3$ $0.65714$ $0.65843$ $0.62658$ $0.62945$ $0.27870$ $0.27961$ $3.6$ $0.68327$ $0.68452$ $0.64684$ $0.64975$ $0.30128$ $0.30210$ $3.9$ $0.70659$ $0.70774$ $0.66455$ $0.66746$ $0.32228$ $0.32310$ $4.2$ $0.72749$ $0.72821$ $0.68014$ $0.68282$ $0.34180$ $0.34302$ $4.5$ $0.74630$ $0.74664$ $0.69397$ $0.69571$ $0.35999$ $0.36042$ $4.8$ $0.76329$ $ 0.70724$ $0.37695$ $ 5.1$ $0.77870$ $ 0.71735$ $0.72349$ $0.39279$ $ 5.4$ $0.79272$ $ 0.72731$ $ 0.40762$ $ 5.7$ $0.80552$ $ 0.73635$ $ 0.43459$ $ 6.0$ $0.81723$ $ 0.74457$ $ 0.43459$ $-$	2.4	0.55661	0.55798	0.54455	0.54709	0.20046	0.20153
3.0 $0.62772$ $0.62905$ $0.60325$ $0.60605$ $0.25442$ $0.25540$ $3.3$ $0.65714$ $0.65843$ $0.62658$ $0.62945$ $0.27870$ $0.27961$ $3.6$ $0.68327$ $0.68452$ $0.64684$ $0.64975$ $0.30128$ $0.30210$ $3.9$ $0.70659$ $0.70774$ $0.66455$ $0.66746$ $0.32228$ $0.32310$ $4.2$ $0.72749$ $0.72821$ $0.68014$ $0.68282$ $0.34180$ $0.34302$ $4.5$ $0.74630$ $0.74664$ $0.69397$ $0.69571$ $0.35999$ $0.36042$ $4.8$ $0.76329$ $ 0.70629$ $0.70724$ $0.37695$ $ 5.1$ $0.77870$ $ 0.72731$ $ 0.40762$ $ 5.7$ $0.80552$ $ 0.73635$ $ 0.42153$ $ 6.0$ $0.81723$ $ 0.74457$ $ 0.43459$ $-$	2.7	0.59444	0.59580	0.57618	0.57888	0.22835	0.22939
3.3 $0.65714$ $0.65843$ $0.62658$ $0.62945$ $0.27870$ $0.27961$ $3.6$ $0.68327$ $0.68452$ $0.64684$ $0.64975$ $0.30128$ $0.30210$ $3.9$ $0.70659$ $0.70774$ $0.66455$ $0.66746$ $0.32228$ $0.32310$ $4.2$ $0.72749$ $0.72821$ $0.68014$ $0.68282$ $0.34180$ $0.34302$ $4.5$ $0.74630$ $0.74664$ $0.69397$ $0.69571$ $0.35999$ $0.36042$ $4.8$ $0.76329$ $ 0.70629$ $0.70724$ $0.37695$ $ 5.1$ $0.77870$ $ 0.71735$ $0.72349$ $0.39279$ $ 5.4$ $0.79272$ $ 0.72731$ $ 0.40762$ $ 5.7$ $0.80552$ $ 0.74457$ $ 0.43459$ $ 6.0$ $0.81723$ $ 0.74457$ $ 0.43459$ $-$	3.0	0.62772	0.62905	0.60325	0.60605	0.25442	0.25540
$3.6$ $0.68327$ $0.68452$ $0.64684$ $0.64975$ $0.30128$ $0.30210$ $3.9$ $0.70659$ $0.70774$ $0.66455$ $0.66746$ $0.32228$ $0.32310$ $4.2$ $0.72749$ $0.72821$ $0.68014$ $0.68282$ $0.34180$ $0.34302$ $4.5$ $0.74630$ $0.74664$ $0.69397$ $0.69571$ $0.35999$ $0.36042$ $4.8$ $0.76329$ $ 0.70629$ $0.70724$ $0.37695$ $ 5.1$ $0.77870$ $ 0.71735$ $0.72349$ $0.39279$ $ 5.4$ $0.79272$ $ 0.72731$ $ 0.40762$ $ 5.7$ $0.80552$ $ 0.74657$ $ 0.42153$ $ 6.0$ $0.81723$ $ 0.74457$ $ 0.43459$ $ \infty$ $1$ $1$ $1$ $1$ $1$ $1$ $1$	3.3	0.65714	0.65843	0.62658	0.62945	0.27870	0.27961
3.9 $0.70659$ $0.70774$ $0.66455$ $0.66746$ $0.32228$ $0.32310$ $4.2$ $0.72749$ $0.72821$ $0.68014$ $0.68282$ $0.34180$ $0.34302$ $4.5$ $0.74630$ $0.74664$ $0.69397$ $0.69571$ $0.35999$ $0.36042$ $4.8$ $0.76329$ $ 0.70629$ $0.70724$ $0.37695$ $ 5.1$ $0.77870$ $ 0.71735$ $0.72349$ $0.39279$ $ 5.4$ $0.79272$ $ 0.72731$ $ 0.40762$ $ 5.7$ $0.80552$ $ 0.74657$ $ 0.42153$ $ 6.0$ $0.81723$ $ 0.74457$ $ 0.43459$ $-$	3.6	0.68327	0.68452	0.64684	0.64975	0.30128	0.30210
$4.2$ $0.72749$ $0.72821$ $0.68014$ $0.68282$ $0.34180$ $0.34302$ $4.5$ $0.74630$ $0.74664$ $0.69397$ $0.69571$ $0.35999$ $0.36042$ $4.8$ $0.76329$ $ 0.70629$ $0.70724$ $0.37695$ $ 5.1$ $0.77870$ $ 0.71735$ $0.72349$ $0.39279$ $ 5.4$ $0.79272$ $ 0.72731$ $ 0.40762$ $ 5.7$ $0.80552$ $ 0.74657$ $ 0.42153$ $ 6.0$ $0.81723$ $ 0.74457$ $ 0.43459$ $ \infty$ $1$ $1$ $1$ $1$ $1$ $1$	3.9	0.70659	0.70774	0.66455	0.66746	0.32228	0.32310
	4.2	0.72749	0.72821	0.68014	0.68282	0.34180	0.34302
$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$	4.5	0.74630	0.74664	0.69397	0.69571	0.35999	0.36042
	4.8	0.76329	_	0.70629	0.70724	0.37695	_
$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$	5.1	0.77870	_	0.71735	0.72349	0.39279	_
$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$	5.4	0.79272	_	0.72731	_	0.40762	_
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	5.7	0.80552	_	0.73635	_	0.42153	_
∞ <u>1</u> <u>1</u> <u>1</u> <u>1</u> <u>1</u> <u>1</u> <u>1</u>	6.0	0.81723	_	0.74457	_	0.43459	_
	$^{\infty}$	1	1	1	1	1	1

Table A1. Cont.

# References

- 1. He, G.K. High-rise building structure facing the 21st century. Build. Sci. 2002, 11, 1–4. (In Chinese)
- 2. Hu, S.D.; Fang, E.H. Analysis of the world's tallest 100 buildings. Constr. Technol. 2004, 35, 540–542. (In Chinese)
- 3. Xiang, H.F. Prospect of world bridge engineering in the 21st century. J. Civ. Eng. 2000, 33, 1–6. (In Chinese)
- 4. Qin, R. Long-Span Bridge Structure; Science Publishing House: Beijing, China, 2008.
- 5. Gere, J.M.; Timoshenko, S.P. Mechanics of Materials; PWS Publishing Company: Boston, MA, USA, 1997.
- 6. Love, A.E.H. The Mathematics Theory of Elasticicy, 4th ed.; New York Dover Publications: Mineola, NY, USA, 1944.
- 7. Frisch-Fay, R. Flexible Bars; Butter Worths: London, UK, 1962.
- 8. Yu, T.X.; Zhang, L.Z. Theory of Plastic Bending and Its Applications; Science Publishing House: Beijing, China, 1992.
- 9. Nayfeh, A.H. Perturbation Methods; Wiley: New York, NY, USA, 2000.
- 10. Novozhilov, V.V. Foundations of the Nonlinear Theory of Elasticity; Graylock Press: New York, NY, USA, 1953.
- 11. Luo, A.C.J. Nonlinear Deformable-Body Dynamics; Higher Education Press: Beijing, China; Springer: Berlin/Heidelberg, Germany, 2010.
- 12. Adomian, G. Nonlinear stochastic differential equations. J. Math. Anal. Appl. 1976, 55, 441–452. [CrossRef]
- 13. Karmishin, A.V.; Zhukov, A.T.; Kolosov, V.G. *Methods of Dynamics Calculation and Testing for Thin-Walled Structures*; Mashinostroyenie: Moscow, Russia, 1990. (In Russian)
- 14. Abbasbandy, S. The application of homotopy analysis method to nonlinear equations arising in heat transfer. *Phys. Lett. A* 2006, 360, 109–113. [CrossRef]
- 15. Liang, S.; Jeffrey, D.J. Approximate solutions to a parameterized sixth order boundary value problem. *Comput. Math. Appl.* **2010**, 59, 247–253. [CrossRef]
- Ghotbi, A.R.; Omidvar, M.; Barari, A. Infiltration in unsaturated soils—An analytical approach. *Comput. Geotech.* 2011, 38, 777–782. [CrossRef]
- 17. Nassar, C.J.; Revelli, J.F.; Bowman, R.J. Application of the homotopy analysis method to the Poisson Boltzmann equation for semiconductor devices. *Commun. Nonlinear Sci. Numer. Simul.* **2011**, *16*, 2501–2512. [CrossRef]
- 18. Aureli, M. A framework for iterative analysis of non-classically damped dynamical systems. *J. Sound Vib.* **2014**, 333, 6688–6705. [CrossRef]
- 19. Sardanyés, J.; Rodrigues, C.; Januário, C.; Martins, N.; Gil-Gómez, G.; Duarte, J. Activation of effector immune cells promotes tumor stochastic extinction: A homotopy analysis approach. *Appl. Math. Comput.* **2015**, 252, 484–495. [CrossRef]

- 20. Zou, K.; Nagarajaiah, S. An analytical method for analyzing symmetry-breaking bifurcation and period-doubling bifurcation. *Commun. Nonlinear Sci. Numer. Simul.* **2015**, *22*, 780–792. [CrossRef]
- Xu, D.; Lin, Z.; Liao, S.; Stiassnie, M. On the steady-state fully resonant progressive waves in water of finite depth. J. Fluid Mech. 2012, 710, 379–418. [CrossRef]
- 22. Liu, Z.; Liao, S.J. Steady-state resonance of mulendle wave interactions in deep water. J. Fluid Mech. 2014, 742, 664–700. [CrossRef]
- Cheng, J.; Zhu, S.P.; Liao, S.J. An explicit series approximation to the optimal exercise boundary of American put options. Commun. Nonlin. Sci. Numer. Simul. 2010, 15, 1148–1158. [CrossRef]
- Liao, S.; Tan, Y. A general approach to obtain series solutions of nonlinear differential equations. *Stud. Appl. Math.* 2007, 119, 297–354. [CrossRef]
- Liao, S.; Zhao, Y. On the method of directly defining inverse mapping for nonlinear differential equations. *Numer. Algor.* 2016, 72, 989–1020. [CrossRef]
- Liao, S.J. Beyond Perturbation: Introduction to the Homotopy Analysis Method; Chapman & Hall/CRC Press: Boca Raton, FL, USA, 2003.
- Sajid, M.; Hayat, T. Comparison of HAM and HPM methods for nonlinear heat conduction and convection equations. *Nonlinear Anal. Real World Appl.* 2008, 9, 2296–2301. [CrossRef]
- Van Gorder, R.A. Analytical method for the construction of solutions to the Föppl–Von Kármán equations governing deflections of a thin flat plate. *Int. J. Non-Linear Mech.* 2012, 47, 1–6. [CrossRef]
- Zhong, X.X.; Liao, S.J. Analytic Solutions of Von Kármán Plate under Arbitrary Uniform Pressure—Equations in Differential Form. *Stud. Appl. Math.* 2017, 138, 371–400. [CrossRef]
- Liu, Z.; Xu, D.L.; Li, J.; Peng, T.; Alsaedi, A.; Liao, S.J. On the existence of steady-state resonant waves in experiments. *J. Fluid Mech.* 2015, 763, 1–23. [CrossRef]
- 31. Liao, S.; Xu, D.; Stiassnie, M. On the steady-state nearly resonant waves. J. Fluid Mech. 2016, 794, 175–199. [CrossRef]
- Wang, J.; Chen, J.-K.; Liao, S. An explicit solution of the large deformation of a cantilever beam under point load at the free end. J. Comput. Appl. Math. 2008, 212, 320–330. [CrossRef]