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Free Vibration Analysis of Laminated Functionally Graded Carbon Nanotube-Reinforced Composite Doubly Curved Shallow Shell Panels Using a New Four-Variable Refined Theory

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Abstract: In this paper, a new four-variable refined shell theory is developed for free vibration analysis of multi-layered functionally graded carbon nanotube-reinforced composite (FG-CNTRC) doubly curved shallow shell panels. The theory has only four unknowns and satisfies zero stress conditions at the free surfaces without correction factor. Five different types of carbon nanotube (CNTs) distribution through the thickness of each FG-CNT layer are considered. Governing equations of simply supported doubly curved FG-CNTRC panels are derived from Hamilton's principle. The resultant eigenvalue system is solved to obtain the frequencies and mode shapes of the anti-symmetric cross-ply laminated panels by using the Navier solution. The numerical results in the comparison examples have proved the accuracy and efficiency of the developed model. Detailed parametric studies have been carried out to reveal the influences of CNTs volume fraction, CNTs distribution, CNTs orientation, dimension ratios and curvature on the free vibration responses of the doubly curved laminated FG-CNTRC panels.

Keywords: free vibration analysis; doubly-curved shell and panel; nano-composites; functionally graded carbon nanotube-reinforced composite (FG-CNTRC); four-variable refined shell theory

1. Introduction

Functionally graded carbon nanotube-reinforced composites were first proposed by Shen [1] and have been widely accepted as a new advanced material. In functionally graded carbon nanotube-reinforced composite (FG-CNTRC) structures, the CNTs are assumed to be distributed and functionally graded with certain rules along the desired direction to improve the mechanical properties of the structures. Due to the curvature effect, doubly curved shell structures possess increased structural stiffness as compared to flat ones. Therefore, doubly curved shells are often employed to fabricate structural elements of modern constructions made of advanced materials in various engineering disciplines such as aerospace, civil, marine and mechanical engineering. It is thus significant and very meaningful to explore the mechanical response of doubly curved shells made of laminated FG-CNTRC.

Due to its simplicity and effectiveness, the equivalent single-layer model is used for multi-layer composite materials. Among the equivalent single layer models, the model based on the classical theory (CPT) [2] only provides accurate results for the thin shell because it completely neglects the effect of shear deformation. To overcome the limitations of CPT, the model based on the first-order shear deformation theory (FSDT) [3] takes into account the shear deformation effects and provides relatively accurate results for both thin and moderately thick shells, but it has to use shear correction

factor. Therefore, the model based on the higher-order shear deformation theory (HSDT) [4–6] is often desirable. However, it is not convenient to use HSDT because the equations of motions based on HSDT are complicated and difficult to solve. Therefore, the development of simple HSDT is needed. In addition to these, a four-variable deformation theory [7–11] has been developed and applied recently. In this theory model, the transverse shear stresses are satisfied to be parabolic and to be zero on free surfaces. Furthermore, it has only four unknowns, thus the governing equations can be reduced to four.

Based on the above-mentioned theories, various studies have been done to investigate the bending, buckling and vibration responses of FG-CNTRC shells and panels. Using the third-order shear deformation theory, Mehrabadi and Aragh [12] investigated static behavior of FG-CNTRC cylindrical shells. Aragh et al. [13] and Yas et al. [14] studied free vibration of FG-CNTRC cylindrical panels. Alibeigloo [15] analyzed the free vibration behavior of the FG-CNTRC cylindrical panel embedded in piezoelectric layers based on the three-dimensional theory of elasticity and the state-space technique. Lei et al. [16] presented the first-known dynamic stability of FG-CNTRC cylindrical panels under static and periodic axial force. Rasool el al. [17] analyzed the stress wave propagation of FG-CNTRC cylinders subjected to an impact load by using an element-free method. In [18], Shen and Zhang investigated thermal post-buckling of FG-CNTRC cylindrical shells subjected to a uniform temperature rise. Based on a HSDT with a von Kármán-type of kinematic nonlinearity, Shen [19] presented the thermal post-buckling and torsional post-buckling of FG-CNTRC cylindrical shells. Furthermore, Shen and Xiang also performed research on nonlinear vibration [20], and post-buckling [21] behavior of FG-CNTRC cylindrical shells in the thermal environment. A post-buckling analysis of FG-CNTRC cylindrical panels subjected to axial compression was also presented by Liew et al. [22]. In this study, Liew et al. used a meshless approach and arc-length method combined with the modified Newton-Raphson method to trace the post-buckling path. Using the element-free kp-Ritz method, Lei et al. [23] investigated free vibration of FG-CNTRC rotating cylindrical panels. Based on the generalized differential quadrature method (GDQM) and the finite element (FE) method, Tornabene et al. [24] and Thomas et al. [25], respectively, investigated free vibration of FG-CNT-reinforced laminated composite doubly curved shells.

The purpose of this paper is to develop a new four-variable refined shell theory for free vibration analysis of multi-layered functionally graded carbon nanotube-reinforced composite doubly curved panels. The present theory has only four unknowns but it satisfies the stress-free boundary conditions on the top and bottom surface without using shear correction factors. The distribution of the carbon nanotube (CNT) through the thickness of each layer may be functionally graded or uniformly distributed. The resultant eigenvalue system is solved to obtain the frequencies and mode shapes of the anti-symmetric, cross-ply laminated panels by Navier solution. The accuracy of the presented formulation is investigated by comparing the obtained natural frequencies with existing results in the literature. Also, a novelty parameter study of the laminated FG-CNTRC doubly-curved panels of which the geometrical parameters, CNTs distributions, the volume fraction of CNTs, as well as the number of layers are also reported in detail.

2. Theoretical Formulations

2.1. Description of the Model

As shown in Figure 1, a doubly curved FG-CNTRC shell panel in the orthogonal curvilinear coordinate system (x, y, z) is considered as the modal analysis. The panel has curvilinear length a in the x-direction, curvilinear width b in the y-direction, thickness h in the z-direction. In the middle surface of the panel, the principal radii of curvature, denoted by R_x and R_y , are assumed as constants. This results in unit Lamé parameters. Here, four special kinds of the doubly curved shell panels are investigated such as plate (PLA, $R_x = R_y = \infty$), cylindrical (CYL) panel ($R_x = R$ and $R_y = \infty$), spherical (SPH) panel ($R_x = R_y$), and hyperbolic paraboloid (HPR) panel ($R_x = -R_y$).

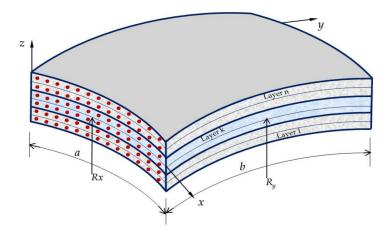


Figure 1. Schematic of the laminated functionally graded carbon nanotube-reinforced composite (FG-CNTRC) doubly curved panel.

2.2. Material Properties of Functionally Graded Carbon Nanotube-Reinforced Composite

In the present study, the lamina is assumed to be perfectly bonded at layer interfaces. As shown in Figure 2, five types of functionally graded distributions of CNTs in each layer are taken into consideration, named as UD, FG-A, FG-V, FG-X and FG-O.

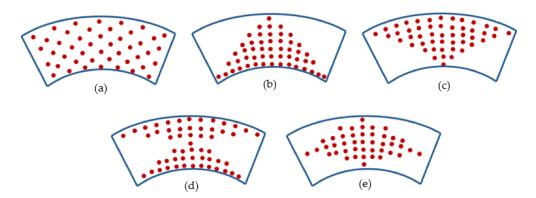


Figure 2. Configurations of the FG-CNTRC panels: (a) UD; (b) FG-A; (c) FG-V; (d) FG-X; (e) FG-O.

For these cases, the CNT volume fractions are given as [26]:

$$\begin{aligned} UD: \ V_{CNT}(z) &= V_{CNT}^{*}; \\ FG - V: \ V_{CNT}(z) &= 2V_{CNT}^{*} \frac{z - z_{k}}{z_{k+1} - z_{k}}; \\ FG - A: \ V_{CNT}(z) &= 2V_{CNT}^{*} \frac{z_{k+1} - z_{k}}{z_{k+1} - z_{k}}; \\ FG - O: \ V_{CNT}(z) &= 2V_{CNT}^{*} \left(1 - \frac{|2z - z_{k} - z_{k+1}|}{z_{k+1} - z_{k}}\right); \end{aligned}$$
(1)
$$\begin{aligned} FG - X: \ V_{CNT}(z) &= 2V_{CNT}^{*} \left(\frac{|2z - z_{k} - z_{k+1}|}{z_{k+1} - z_{k}}\right); \end{aligned}$$

where z_k and z_{k+1} are the coordinates of the *k*-th layer to the reference plane (z = 0). V_{CNT}^* is the given volume fraction of CNTs and can be calculated as:

$$V_{CNT}^{*} = \frac{w_{CNT}}{w_{CNT} + (\rho^{CNT} / \rho^{m}) - (\rho^{CNT} / \rho^{m}) w_{CNT}}$$
(2)

in which, w_{CNT} is the mass fraction of the carbon nanotube, ρ^m and ρ^{CNT} are mass densities of the matrix and the CNT, respectively. The effective material properties of FG-CNTRC of each layer can be expressed by the extended rule of the mixture as follows [27]:

$$E_{11}(z) = \eta_1 V_{CNT}(z) E_{11}^{CNT} + V_m(z) E^m$$

$$\frac{\eta_2}{E_{22}(z)} = \frac{V_{CNT}(z)}{E_{22}^{CNT}} + \frac{V_m(z)}{E^m};$$

$$\frac{\eta_3}{G_{12}(z)} = \frac{V_{CNT}(z)}{G_{12}^{CNT}} + \frac{V_m(z)}{G^m};$$

$$\rho(z) = V_{CNT}(z)\rho^{CNT} + V_m(z)\rho^m;$$

$$v_{12} = V_{CNT}^* v_{12}^{CNT} + V_m(z)v^m$$
(3)

where E_{11}^{CNT} , E_{22}^{CNT} , E^m and G_{12}^{CNT} , G^m are the Young's moduli and shear modulus of CNT and matrix; η_1, η_2 and η_3 are CNT/matrix efficiency parameters; $V_{CNT}(z)$ and $V_m(z)$ are volume fractions of CNT and matrix, and are related by $V_{CNT}(z) + V_m(z) = 1$; v_{12}^{CNT} and v^m are Poisson's ratio of CNT and matrix.

2.3. Kinematic Relations

This work aims to establish a new shear deformation shell theory. The main idea of the present theory comes from the four-variable refined theory [8,9,11,28,29]. According to assumptions of various four-variable refined theories, the transverse displacement w is partitioned into the bending component w_b and shear component w_s , the in-plane displacements u and v are also partitioned into the extension component u_0 , v_0 , the bending component u_b , v_b , and shear component u_s , v_s . Therefore, the displacement field in the doubly curved shell space can be expressed as follows:

$$u(x, y, z, t) = \left(1 + \frac{z}{R_x}\right)u_0(x, y, t) - z\frac{\partial w_b(x, y, t)}{\partial x} - f(z)\frac{\partial w_s(x, y, t)}{\partial x}$$

$$v(x, y, z, t) = \left(1 + \frac{z}{R_y}\right)v_0(x, y, t) - z\frac{\partial w_b(x, y, t)}{\partial y} - f(z)\frac{\partial w_s(x, y, t)}{\partial y}$$

$$w(x, y, z, t) = w_b(x, y, t) + w_s(x, y, t)$$
(4)

where u_0 , v_0 denote the displacements along x and y coordinate directions of the corresponding point on the reference surface; w_b and w_s are the bending and shear components of the transverse displacement, respectively; f(z) represents shape function determining the distribution of the transverse shear strains and stresses along the thickness. By the same methodology, in the previous study [29], we proposed a new shape function f(z) as follows:

$$f(z) = z \left[-\frac{1}{8} + \frac{3}{2} \left(\frac{z}{h} \right)^2 \right]$$
(5)

Detail steps to construct this shape function for shell panels are listed in Appendix A. The strains associated with the displacement field in Equation (4) are:

$$\begin{aligned} \varepsilon_{xx} &= \frac{1}{1+z/R_x} \Big[\varepsilon_x^0 + z\kappa_x^b + f(z)\kappa_x^s \Big];\\ \varepsilon_{yy} &= \frac{1}{1+z/R_y} \Big[\varepsilon_y^0 + z\kappa_y^b + f(z)\kappa_y^s \Big];\\ \gamma_{xy} &= \frac{1}{1+z/R_x} \Big[\gamma_{xy}^0 + z\kappa_{xy}^b + f(z)\kappa_{xy}^s \Big] + \frac{1}{1+z/R_y} \Big[\gamma_{yx}^0 + z\kappa_{yx}^b + f(z)\kappa_{yx}^s \Big];\\ \gamma_{xz} &= \frac{1}{1+z/R_x} g(z)\gamma_{xz}^s;\\ \gamma_{yz} &= \frac{1}{1+z/R_y} g(z)\gamma_{yz}^s \end{aligned}$$
(6)

where:

$$\varepsilon_{x}^{0} = \left(\frac{\partial u_{0}}{\partial x} + \frac{w_{b}}{R_{x}} + \frac{w_{s}}{R_{x}}\right); \quad \gamma_{xy}^{0} = \frac{\partial v_{0}}{\partial x};$$

$$\varepsilon_{y}^{0} = \left(\frac{\partial v_{0}}{\partial y} + \frac{w_{b}}{R_{y}} + \frac{w_{s}}{R_{y}}\right); \quad \gamma_{yx}^{0} = \frac{\partial u_{0}}{\partial y};$$

$$k_{x}^{b} = \left(\frac{1}{R_{x}}\frac{\partial u_{0}}{\partial x} - \frac{\partial w_{b}^{2}}{\partial x^{2}}\right); \quad \kappa_{y}^{b} = \left(\frac{1}{R_{y}}\frac{\partial v_{0}}{\partial y} - \frac{\partial w_{b}^{2}}{\partial y^{2}}\right);$$

$$\kappa_{xy}^{b} = \left(\frac{1}{R_{y}}\frac{\partial v_{0}}{\partial x} - \frac{\partial w_{b}^{2}}{\partial x \partial y}\right); \quad \kappa_{yx}^{b} = \left(\frac{1}{R_{x}}\frac{\partial u_{0}}{\partial y} - \frac{\partial w_{b}^{2}}{\partial x \partial y}\right);$$

$$k_{x}^{s} = -\frac{\partial^{2}w_{s}}{\partial x^{2}}; \quad k_{y}^{s} = -\frac{\partial^{2}w_{s}}{\partial y^{2}}; \quad \kappa_{xy}^{s} = \frac{\partial^{2}w_{s}}{\partial x \partial y}; \quad \kappa_{yx}^{s} = -\frac{\partial^{2}w_{s}}{\partial x \partial y};$$

$$\gamma_{xz} = \frac{1}{1+z/R_{x}}g(z)\frac{\partial w_{s}}{\partial x}; \quad g(z) = (1-f'(z));$$

$$\gamma_{yz} = \frac{1}{1+z/R_{x}}g(z)\frac{\partial w_{s}}{\partial y}$$

The constitutive relation for an individual layer can be determined by the generalized Hooke's law, namely [30,31]: $\begin{bmatrix} -k & -k \\ -k & -k \end{bmatrix}$

$$\begin{cases} \sigma_{xx}^{k} \\ \sigma_{yy}^{k} \\ \tau_{yz}^{k} \\ \tau_{xy}^{k} \\ \tau_{xy}^{k} \end{cases} = \begin{bmatrix} \overline{Q}_{11}^{k} & \overline{Q}_{11}^{k} & 0 & 0 & \overline{Q}_{16}^{k} \\ \overline{Q}_{12}^{k} & \overline{Q}_{22}^{k} & 0 & 0 & \overline{Q}_{26}^{k} \\ 0 & 0 & \overline{Q}_{44}^{k} & \overline{Q}_{45}^{k} & 0 \\ 0 & 0 & \overline{Q}_{45}^{k} & \overline{Q}_{55}^{k} & 0 \\ 0 & 0 & \overline{Q}_{45}^{k} & \overline{Q}_{55}^{k} & 0 \\ \overline{Q}_{16}^{k} & \overline{Q}_{26}^{k} & 0 & 0 & \overline{Q}_{66}^{k} \end{bmatrix} \begin{pmatrix} \varepsilon_{xx} \\ \varepsilon_{yy} \\ \gamma_{yz} \\ \gamma_{xz} \\ \gamma_{xy} \end{pmatrix}$$
(8)

where \overline{Q}_{ij}^k are the transformed material constraints expressed in terms of material constants:

$$\overline{Q}_{11}^{k} = Q_{11}\cos^{4}\theta^{k} + 2(Q_{12} + 2Q_{66})\sin^{2}\theta^{k}\cos^{2}\theta^{k} + Q_{22}\sin^{4}\theta^{k};$$

$$\overline{Q}_{12}^{k} = (Q_{11} + Q_{22} - 4Q_{66})\sin^{2}\theta^{k}\cos^{2}\theta^{k} + Q_{12}(\sin^{4}\theta^{k} + \cos^{4}\theta^{k});$$

$$\overline{Q}_{22}^{k} = Q_{11}\sin^{4}\theta^{k} + 2(Q_{12} + 2Q_{66})\sin^{2}\theta^{k}\cos^{2}\theta^{k} + Q_{22}\cos^{4}\theta^{k};$$

$$\overline{Q}_{16}^{k} = (Q_{11} - Q_{12} - 2Q_{66})\sin\theta^{k}\cos^{3}\theta^{k} + (Q_{12} - Q_{22} + 2Q_{66})\sin^{3}\theta^{k}\cos\theta^{k};$$

$$\overline{Q}_{26}^{k} = (Q_{11} - Q_{12} - 2Q_{66})\sin^{3}\theta^{k}\cos\theta^{k} + (Q_{12} - Q_{22} + 2Q_{66})\sin\theta^{k}\cos^{3}\theta^{k};$$

$$\overline{Q}_{66}^{k} = (Q_{11} + Q_{22} - 2Q_{12} - 2Q_{66})\sin^{2}\theta^{k}\cos^{2}\theta^{k} + Q_{66}(\sin^{4}\theta^{k} + \cos^{4}\theta^{k});$$

$$\overline{Q}_{44}^{k} = Q_{44}\cos^{2}\theta^{k} + Q_{55}\sin^{2}\theta^{k};$$

$$\overline{Q}_{55}^{k} = Q_{55}\cos^{2}\theta^{k} + Q_{44}\sin^{2}\theta^{k}.$$
(9)

in which, Q_{ij} are the plane stress-reduced stiffnesses defined in terms of the engineering constants in the material axes of the layer. For each CNT layer:

$$Q_{11} = \frac{E_{11}(z)}{1 - \nu_{12}\nu_{21}}; Q_{12} = \frac{\nu_{12}E_{22}(z)}{1 - \nu_{12}\nu_{21}}; Q_{22} = \frac{E_{22}(z)}{1 - \nu_{12}\nu_{21}};$$

$$Q_{44} = G_{23}(z); Q_{55} = G_{13}(z); Q_{66} = G_{12}(z)$$
(10)

2.4. Governing Equations

Hamilton's principle is used herein to derive the equations of motion. In the absence of external forces, the principle can be stated in the analytical form as [32]:

$$\int_{t_1}^{t_2} (\delta U - \delta K) dt = 0 \tag{11}$$

where δU is the variation of the strain energy, δK is the variation of the kinetic energy, t_1 and t_2 are arbitrary time variables. The strain energy of the plate can be calculated as:

$$U = \frac{1}{2} \int_{0}^{a} \int_{0}^{b} \int_{-h/2}^{h/2} \left(\sigma_{xx} \varepsilon_{xx} + \sigma_{yy} \varepsilon_{yy} + \tau_{xy} \gamma_{xy} + \tau_{xz} \gamma_{xz} + \tau_{yz} \gamma_{yz} \right) \left(1 + \frac{z}{R_x} \right) \left(1 + \frac{z}{R_y} \right) dz dy dx$$

$$= \frac{1}{2} \int_{0}^{a} \int_{0}^{b} \left(N_{xx} \varepsilon_{xx}^{0} + N_{yy} \varepsilon_{yy}^{0} + N_{xy} \gamma_{xy}^{0} + N_{yx} \gamma_{yx}^{0} + M_{xx}^{b} \kappa_{xx}^{b} + M_{yy}^{b} \kappa_{yy}^{b} + M_{xy}^{b} \kappa_{yx}^{b} + M_{yx}^{b} \kappa_{yx}^{b}$$

where stress resultants (*N*, *M* and *Q*) are defined by:

$$\begin{cases} N_{xx} \\ N_{xy} \\ Q_{xs} \end{cases} = \sum_{k=1}^{n} \int_{z_{k}}^{z_{k+1}} \left(1 + \frac{z}{R_{y}}\right) \begin{cases} \sigma_{xx}^{k} \\ \sigma_{xy}^{k} \\ \tau_{xz}^{k} \end{cases} dz; \begin{cases} N_{yy} \\ N_{yx} \\ Q_{ys} \end{cases} = \sum_{k=1}^{n} \int_{z_{k}}^{z_{k+1}} \left(1 + \frac{z}{R_{y}}\right) \begin{cases} \sigma_{xx}^{k} \\ \sigma_{xy}^{k} \\ \tau_{xz}^{k} \end{cases} dz; \begin{cases} M_{yy}^{b} \\ M_{yy}^{b} \end{cases} = \sum_{k=1}^{n} \int_{z_{k}}^{z_{k+1}} \left(1 + \frac{z}{R_{y}}\right) \begin{cases} \sigma_{xx}^{k} \\ \sigma_{xy}^{k} \end{cases} dz; \begin{cases} M_{yy}^{b} \\ M_{yx}^{b} \end{cases} = \sum_{k=1}^{n} \int_{z_{k}}^{z_{k+1}} \left(1 + \frac{z}{R_{y}}\right) \begin{cases} \sigma_{xx}^{k} \\ \sigma_{xy}^{k} \end{cases} dz; \begin{cases} M_{yy}^{b} \\ M_{yx}^{b} \end{cases} = \sum_{k=1}^{n} \int_{z_{k}}^{z_{k+1}} \left(1 + \frac{z}{R_{y}}\right) \begin{cases} \sigma_{xx}^{k} \\ \sigma_{xy}^{k} \end{cases} dz; \end{cases} (13)$$

Based on the constitutive relations (8), strain-displacement relation (6) and displacement field (4), the force and moment resultants can be rewritten in terms of displacement components as:

$$\begin{pmatrix} N_{xx} \\ N_{yy} \\ N_{xy} \\ N_{xy} \\ N_{xy} \\ N_{yx} \\ M_{yy} \\ M_{xx} \\ M_{yy}^{b} \\ M_{xx}^{b} \\ M_{yy}^{b} \\ M_{xx}^{b} \\ M_{yy}^{b} \\ M_{xy}^{b} \\ M_{yy}^{b} \\ M_{yx}^{b} \\ M_{yy}^{b} \\ M_{yy}$$

$$\left\{\begin{array}{c}Q_{ys}\\Q_{xs}\end{array}\right\} = \left[\begin{array}{c}A_{44}^s & A_{45}^s\\A_{45}^s & \overline{A}_{55}^s\end{array}\right] \left\{\begin{array}{c}\gamma_{yz}^s\\\gamma_{xz}^s\end{array}\right\}$$
(15)

in which:

$$\begin{cases} A_{ij}, B_{ij}, D_{ij}, B_{ij}^{s}, D_{ij}^{s}, A_{ij}^{s} \end{cases} = \sum_{1}^{N} \int_{z_{k}}^{z_{k+1}} \overline{Q}_{ij}^{(k)} \{1, z, z^{2}, f(z), zf(z), g^{2}(z)\} dz; \\ \{\overline{A}_{ij}, \overline{B}_{ij}, \overline{D}_{ij}, \overline{B}_{ij}^{s}, \overline{D}_{ij}^{s}, \overline{A}_{ij}^{s} \} = \{A_{ijx}, B_{ijx}, D_{ijx}, B_{ijx}^{s}, D_{ijx}^{s}, A_{ijx}^{s} \} + \frac{\{B_{ijx}, D_{ijx}, E_{ijx}, D_{ijx}^{s}, E_{ijx}^{s}, AA_{ijx}^{s} \}}{R_{y}}; \\ \begin{cases} A_{ijx}, B_{ijx}, D_{ijx}, E_{ijx}, B_{ijx}^{s}, D_{ijx}^{s}, E_{ijx}^{s}, A_{ijx}^{s} \} + \frac{\{A_{ijx}, B_{ijx}, D_{ijx}, E_{ijx}, AA_{ijx}^{s} \}}{R_{y}} \} \\ \sum_{1}^{N} \int_{z_{k}}^{z_{k+1}} \overline{Q}_{ij}^{(k)} \frac{\{1.z, z^{2}, z^{3}, f(z), zf(z), z^{2}f(z), g(z), zg(z)\}}{1 + z/R_{x}} dz \end{cases} dz$$

$$\begin{cases} A_{ij}, \hat{B}_{ij}, \hat{D}_{ij}, \hat{B}_{ij}^{s}, \hat{D}_{ij}^{s}, \hat{A}_{ij}^{s} \} = \{A_{ijy}, B_{ijy}, D_{ijy}, B_{ijy}^{s}, D_{ijy}^{s}, A_{ijy}^{s} \} + \frac{\{B_{ijy}, D_{ijy}, E_{ijy}, D_{ijy}^{s}, E_{ijy}^{s}, AA_{ijy}^{s} \}}{R_{x}} \end{bmatrix} \\ = \sum_{1}^{N} \int_{z_{k}}^{z_{k+1}} \overline{Q}_{ij}^{(k)} \frac{\{1.z, z^{2}, z^{3}, f(z), zf(z), z^{2}f(z), g(z), zg(z)\}}{1 + z/R_{y}}} dz$$

$$\begin{cases} A_{ijy}, B_{ijy}, D_{ijy}, E_{ijy}, B_{ijy}^{s}, D_{ijy}^{s}, E_{ijy}^{s}, A_{ijy}^{s} \} + \frac{\{B_{ijy}, D_{ijy}, E_{ijy}, AA_{ijy}^{s} \}}{R_{x}} \end{bmatrix} \\ = \sum_{1}^{N} \int_{z_{k}}^{z_{k+1}} \overline{Q}_{ij}^{(k)} \frac{\{1.z, z^{2}, z^{3}, f(z), zf(z), z^{2}f(z), g(z), zg(z)\}}{1 + z/R_{y}}} dz \end{cases}$$

The variation of the kinetic energy of the panel can be written as:

$$K = \frac{1}{2} \int_{0}^{a} \int_{0}^{b} \int_{-h/2}^{h/2} \rho(z) (\dot{u}^{2} + \dot{v}^{2} + \dot{w}^{2}) (1 + \frac{z}{R_{x}}) (1 + \frac{z}{R_{y}}) dz dy dx$$

$$= \frac{1}{2} \int_{0}^{a} \int_{0}^{b} ((\bar{I}_{0}\dot{u}_{0} + \bar{I}_{2}\dot{\phi}_{xb}^{2} + \bar{K}_{1}\dot{\phi}_{xs}^{2} + 2\bar{I}_{1}\dot{u}_{0}\dot{\phi}_{xb} + 2\bar{J}_{1}\dot{u}_{0}\dot{\phi}_{xs} + 2\bar{J}_{2}\dot{\phi}_{xb}\dot{\phi}_{xs} + \bar{I}_{0}\dot{v}_{0}^{2} + \bar{I}_{2}\dot{\phi}_{yb}^{2}$$

$$+ \overline{K}_{1}\dot{\phi}_{ys}^{2} + 2\bar{I}_{1}\dot{v}_{0}\dot{\phi}_{yb} + 2\bar{J}_{1}\dot{v}_{0}\dot{\phi}_{ys} + 2\bar{J}_{2}\dot{\phi}_{yb}\dot{\phi}_{ys} + \bar{I}_{0}(\dot{w}_{b}^{2} + \dot{w}_{2}^{s} + 2\dot{w}_{b}\dot{w}_{s}))dydx$$

$$(17)$$

where:

$$\phi_{xb} = \left(\frac{u_0}{R_x} - \frac{\partial w_b}{\partial x}\right); \ \phi_{xs} = -\frac{\partial w_s}{\partial x}; \ \phi_{yb} = \left(\frac{v_0}{R_y} - \frac{\partial w_b}{\partial y}\right); \ \phi_{ys} = -\frac{\partial w_s}{\partial y}$$
(18)

and $\rho(z)$ is the mass density, and the mass moments of inertia \overline{I}_i (i = 0, 1, 2) are defined as [30,33]:

$$\overline{I}_{i} = I_{i} + I_{i+1} \left(\frac{1}{R_{x}} + \frac{1}{R_{y}} \right) + \frac{I_{i+2}}{R_{x}R_{y}};$$

$$\{I_{0}, I_{1}, I_{2}, I_{3}\} = \sum_{\substack{k=1\\ k=1}}^{N} \int_{z_{k}}^{z_{+1}} \rho(z) \{1, z, z^{2}, z^{3}\} dz;$$

$$\overline{J}_{i} = f(z)\overline{I}_{i-1}; \ \overline{K}_{1} = f^{2}(z)\overline{I}_{0}$$
(19)

Substituting the expressions of U and K from Equation (12) and Equation (17) into Equation (11), and by performing some mathematical manipulations, the equations of motion of the shell panel are obtained as follows:

$$0 = -\int_{A} \begin{bmatrix} \frac{\partial N_{xx}}{\partial x} + \frac{\partial N_{yx}}{\partial y} + \frac{Q_{xb}}{R_x} - \bar{I}_0 \ddot{u}_0 - \bar{I}_1 \left(\frac{\ddot{u}_0}{R_x} - \frac{\partial \ddot{w}_b}{\partial x}\right) + \bar{J}_1 \frac{\partial \ddot{w}_s}{\partial x}}{\partial x} \end{bmatrix} \delta u_0 \\ \begin{bmatrix} \frac{\partial N_{yy}}{\partial y} + \frac{\partial N_{yx}}{\partial x} + \frac{Q_{yb}}{R_y} - \bar{I}_0 \ddot{v}_0 - \bar{I}_1 \left(\frac{\ddot{v}_0}{R_y} - \frac{\partial \ddot{w}_b}{\partial y}\right) + \bar{J}_1 \frac{\partial \ddot{w}_s}{\partial y}}{\partial y} \end{bmatrix} \delta v_0 \\ \begin{bmatrix} -\frac{\partial N_{xx}}{R_x} - \frac{\partial N_{yy}}{R_y} + \frac{\partial Q_{xb}}{\partial x} + \frac{\partial Q_{yb}}{\partial y} - \bar{I}_0 (\ddot{w}_b + \ddot{w}_s)} \\ -\frac{\partial N_{xx}}{R_x} - \frac{\partial N_{yy}}{R_y} + \frac{\partial Q_{xs}}{\partial x} + \frac{\partial Q_{ys}}{\partial y} - \bar{I}_0 (\ddot{w}_b + \ddot{w}_s)} \end{bmatrix} \delta w_b \\ + \int_0^b [\Gamma_x]_0^a dy + \int_0^a [\Gamma_y]_0^b dx \end{aligned}$$
(20)

where:

$$Q_{xb} = \frac{\partial M_{xx}^b}{\partial x} + \frac{\partial M_{yx}^b}{\partial y} - \left(\overline{I}_1 + \frac{\overline{I}_2}{R_x}\right) \ddot{u}_0 + \overline{I}_2 \frac{\partial \ddot{w}_b}{\partial x} + \overline{J}_2 \frac{\partial \ddot{w}_s}{\partial x}$$

$$Q_{yb} = \frac{\partial M_{yy}^b}{\partial y} + \frac{\partial M_{xy}^b}{\partial x} - \left(\overline{I}_1 + \frac{\overline{I}_2}{R_y}\right) \ddot{v}_0 + \overline{I}_2 \frac{\partial \ddot{w}_b}{\partial y} + \overline{J}_2 \frac{\partial \ddot{w}_s}{\partial y}$$

$$\overline{Q}_{xs} = \frac{\partial M_{xx}^s}{\partial x} + \frac{\partial M_{yx}^s}{\partial y} + Q_{xs} - \left(\overline{J}_1 + \frac{\overline{I}_2}{R_x}\right) \ddot{u}_0 + \overline{J}_2 \frac{\partial \ddot{w}_b}{\partial x} + \overline{K}_1 \frac{\partial \ddot{w}_s}{\partial x}$$

$$\overline{Q}_{ys} = \frac{\partial M_{yy}^s}{\partial y} + \frac{\partial M_{xy}^s}{\partial x} + Q_{ys} - \left(\overline{J}_1 + \frac{\overline{I}_2}{R_y}\right) \ddot{v}_0 + \overline{J}_2 \frac{\partial \ddot{w}_b}{\partial x} + \overline{K}_1 \frac{\partial \ddot{w}_s}{\partial y}$$
(21)

and Γ_x , Γ_y are boundary expressions:

$$\Gamma_{x} = \overline{N}_{xx}\delta u_{0} + \overline{N}_{xy}\delta v_{0} + Q_{xb}\delta w_{b} + \overline{Q}_{xs}\delta w_{s} + M^{b}_{xx}\delta\widetilde{\phi}_{xb} + M^{b}_{xy}\delta\widetilde{\phi}_{yb} + M^{s}_{xx}\delta\phi_{xs} + M^{s}_{xy}\delta\phi_{ys}$$

$$\Gamma_{y} = \overline{N}_{yy}\delta v_{0} + \overline{N}_{yx}\delta u_{0} + Q_{yb}\delta w_{b} + \overline{Q}_{ys}\delta w_{s} + M^{b}_{yy}\delta\widetilde{\phi}_{yb} + M^{b}_{yx}\delta\widetilde{\phi}_{xb} + M^{s}_{yy}\delta\phi_{ys} + M^{s}_{yx}\delta\phi_{xs}$$

$$(22)$$

in which:

$$\overline{N}_{xx} = \left(N_{xx} - \frac{M_{xx}^b}{R_x}\right); \ \overline{N}_{xy} = \left(N_{xy} - \frac{M_{xy}^b}{R_y}\right); \ \overline{N}_{yy} = \left(N_{yy} - \frac{M_{yy}^b}{R_y}\right); \ \overline{N}_{yx} = \left(N_{yx} - \frac{M_{yx}^b}{R_x}\right);$$

$$\widetilde{\phi}_{xb} = -\frac{\partial w_b}{\partial x}; \ \phi_{xs} = -\frac{\partial w_s}{\partial x}; \ \widetilde{\phi}_{yb} = -\frac{\partial w_b}{\partial y}; \ \phi_{ys} = -\frac{\partial w_s}{\partial y}$$
(23)

By setting the coefficients of the virtual displacements δu_0 , δv_0 , δw_b , δw_s to zeros, the governing equations are obtained as follows:

$$\delta u_{0} : \frac{\partial N_{xx}}{\partial x} + \frac{\partial N_{yx}}{\partial y} + \frac{Q_{xb}}{R_{x}} = \bar{I}_{0}\ddot{u}_{0} + \bar{I}_{1}\left(\frac{\ddot{u}_{0}}{R_{x}} - \frac{\partial\ddot{w}_{b}}{\partial y}\right) - \bar{J}_{1}\frac{\partial\ddot{w}_{s}}{\partial x}$$

$$\delta v_{0} : \frac{\partial N_{yy}}{\partial y} + \frac{\partial N_{yx}}{\partial x} + \frac{Q_{yb}}{R_{y}} = \bar{I}_{0}\ddot{v}_{0} + \bar{I}_{1}\left(\frac{\ddot{v}_{0}}{R_{y}} - \frac{\partial\ddot{w}_{b}}{\partial y}\right) - \bar{J}_{1}\frac{\partial\ddot{w}_{s}}{\partial y}$$

$$\delta w_{b} : \frac{\partial N_{xx}}{R_{x}} + \frac{\partial N_{yy}}{R_{y}} - \frac{\partial Q_{xb}}{\partial x} - \frac{\partial Q_{yb}}{\partial y} = -\bar{I}_{0}\left(\ddot{w}_{b} + \ddot{w}_{s}\right)$$

$$\delta w_{s} : \frac{\partial N_{xx}}{R_{x}} + \frac{\partial N_{yy}}{R_{y}} - \frac{\partial Q_{xs}}{\partial x} - \frac{\partial Q_{ys}}{\partial y} = -\bar{I}_{0}\left(\ddot{w}_{b} + \ddot{w}_{s}\right)$$
(24)

2.5. Solution Procedure

The Navier method is employed to formulate the closed-form solution for vibration problems of simply supported anti-symmetric cross-ply laminated FG-CNTRC panels. The simply supported boundary conditions on all four edges can be considered as:

$$v_0 = w_b = w_s = w_{b,y} = w_{s,y} = N_{xx} = M^b_{xx} = M^s_{xx} = 0 \text{ at } x = 0 \text{ and } x = a$$
 (25)

$$u_0 = w_b = w_s = w_{b,x} = w_{s,x} = N_{yy} = M^b_{yy} = M^s_{yy} = 0 \text{ at } y = 0 \text{ and } y = b$$
(26)

These boundary conditions are exactly satisfied by the following double Fourier series forms:

$$u(x, y, t) = \sum_{m=1}^{\infty} \sum_{n=1}^{\infty} U_{mn} e^{i\omega t} \cos \alpha_m x \sin \beta_n y;$$

$$v(x, y, t) = \sum_{m=1}^{\infty} \sum_{n=1}^{\infty} V_{mn} e^{i\omega t} \sin \alpha_m x \cos \beta_n y;$$

$$w_b(x, y, t) = \sum_{m=1}^{\infty} \sum_{n=1}^{\infty} W_{bmn} e^{i\omega t} \sin \alpha_m x \sin \beta_n y;$$

$$w_s(x, y, t) = \sum_{m=1}^{\infty} \sum_{n=1}^{\infty} W_{smn} e^{i\omega t} \sin \alpha_m x \sin \beta_n y.$$
(27)

where $(U_{mn}, V_{mn}, W_{bmn}, W_{smn})$ are unknown coefficients to be determined, ω is the circular frequency of vibration, and $i = \sqrt{-1}$, $\alpha_m = m\pi/a$, $\beta_n = n\pi/b$ and m, n denote the number of haft-waves in the x and y directions, respectively.

Substituting the admissible displacement functions of Equation (27) into the equation of motion, Equation (20), one obtains the analytical solution in the following matrix form:

$$\begin{pmatrix} s_{11} & s_{12} & s_{13} & s_{14} \\ s_{12} & s_{22} & s_{23} & s_{24} \\ s_{13} & s_{23} & s_{33} & s_{34} \\ s_{14} & s_{24} & s_{34} & s_{44} \end{pmatrix} - \omega^2 \begin{bmatrix} m_{11} & m_{12} & m_{13} & m_{14} \\ m_{12} & m_{22} & m_{23} & m_{24} \\ m_{13} & m_{23} & m_{33} & m_{34} \\ m_{14} & m_{24} & m_{34} & m_{44} \end{bmatrix} \begin{pmatrix} U_{mn} \\ V_{mn} \\ W_{bmn} \\ W_{smn} \end{pmatrix} = \begin{cases} 0 \\ 0 \\ 0 \\ 0 \end{pmatrix}$$
(28)

where the matrix elements of Equation (28) are given in the Appendix B.

3. Numerical Results and Discussions

In this section, several examples are presented and discussed to verify the accuracy and efficiency of the proposed theory in free vibration analysis of simply supported FG-CNTRC doubly-curved panels. Furthermore, the effects of volume fraction of CNTs, distribution type of CNTs, number of layers, CNT fiber orientation and geometrical parameters on the natural frequencies of panels are also investigated in detail. The material properties for the matrix and CNT are given in Table 1 [34,35]. Also, the CNT efficiency parameters η_j (j = 1,2,3) associated with a given volume fraction V_{CNT}^* are: $\eta_1 = 0.149$ and $\eta_2 = \eta_3 = 0.934$ for the case of $V_{CNT}^* = 0.11$; $\eta_1 = 0.150$ and $\eta_2 = \eta_3 = 0.941$ for the case of $V_{CNT}^* = 0.14$; $\eta_1 = 0.149$ and $\eta_2 = \eta_3 = 1.381$ for the case of $V_{CNT}^* = 0.17$.

Table 1. Material	properties of carbon	nanotube (CNT)	and matrix materials.
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CNT	Matrix
$E_{11}^{CNT} = 5.6466 \ TPa$	$E^m = 2.1 GPa$
$E_{22}^{CNT} = 7.0800 TPa$ $G_{12}^{CNT} = 1.9445 TPa$	$v^m = 0.34$
$G_{12}^{CNT} = 1.9445 \ TPa$	$\rho^m = 1150 kg/m^3$
$v_{12}^{CNT} = 0.175$	-
$\rho^{CNT} = 1400 kg/m^3$	-

3.1. Comparison Studies

To verify the reliability and accuracy of the present model, several comparison studies were carried out with the results of the previous literature [34,35].

Example 1: Free Vibration of the Simply Supported Doubly Curved FG-CNTRC Panels

Free vibration of the simply supported doubly curved single-layered FG-CNTRC panels is further analyzed for the comparison of the results obtained from the present formulation with the existing results developed by Pouresmaeeli and Fazelzadeh [34] based on FSDT formulations. The geometrical dimensions of the panels are taken as a/b = 1 and a/h = 20. Values of material parameters are listed

in Table 1. From the results presented in Table 2, it is observed that the values of the fundamental frequency for plates, spherical, cylindrical, and hyperbolic paraboloid panels have excellent agreement with the available data.

a/R_x b/R_y	V*	UD		FG-V		FG-X		FG-O		
	∂/K_y	V* _{CNT}	[34]	Present	[34]	Present	[34]	Present	[34]	Present
		0.11	20.238	20.087	18.543	17.917	22.432	22.752	17.140	16.653
0.5	0.5	0.14	21.655	21.700	19.779	19.184	23.997	24.642	18.267	17.790
	0.17	25.051	24.691	22.951	21.848	27.883	28.023	21.212	20.419	
0.5 -0.5	0.11	17.106	17.282	14.809	14.617	19.588	20.253	13.364	13.202	
	0.14	18.626	19.005	16.181	16.065	21.225	22.203	14.610	14.493	
	0.17	21.095	21.214	18.225	17.875	24.274	24.879	16.389	16.149	
0.5 0	0.11	18.126	18.210	16.060	15.698	20.548	21.120	14.553	14.297	
	0.14	19.628	19.890	17.391	17.065	22.179	23.044	15.766	15.525	
	0.17	22.380	22.328	19.799	19.111	25.488	25.925	17.903	17.472	
	0.11	18.008	18.201	15.701	15.55	20.624	21.332	14.068	13.907	
0	0 0	0.14	19.608	20.016	17.147	17.089	22.349	23.391	15.378	15.265
	0.17	22.207	22.343	19.315	19.021	25.557	26.208	17.252	17.012	

Table 2. Comparison of the non-dimensional frequencies $\overline{\omega} = \omega (a^2/h) \sqrt{\rho^m / E^m}$ of the simply supported doubly curved FG-CNTRC panels.

3.2. Parametric Studies

In this section, some new results for free vibration of the anti-symmetric cross-ply laminated FG-CNTRC doubly curved shell panels are investigated with respect to FG-CNTRC parameters, curvature, R_x/R_y ratio, aspect *a/b* ratio, and number of layers. The material properties for the matrix and CNT are shown in Table 1.

3.2.1. Effect of FG-CNTRC Parameters

To understand the effect of FG-CNTRC parameters on the free vibration response of different shell panels, non-dimensional frequencies $\overline{\omega}$ of anti-symmetric cross-ply laminated FG-CNTRC doubly curved shell panels with different CNT distribution, CNT volume fraction, and number of CNT layers are examined.

It is observed from Table 3, that the FG-X panels have the highest value of frequency, whereas, the FG-O panels have the lowest one. Therefore, it can be concluded that the type of CNT distribution has a remarkable influence on the stiffness of the FG-CNTRC shell panels. In detail, the CNTs distributed close to the top and bottom surfaces of each FG-CNTRC layer are more efficient than those distributed near the mid-plane of each FG-CNTRC layer in increasing the stiffness of the laminated FG-CNTRC shell panels. This is compatible with conclusions in previous studies in the literature. According to the detailed results, the values of $\overline{\omega}$ can be increased by more than 24% with only 6% increasing CNT volume fraction V_{CNT}^* for any other parameters. Thus, by adjusting a small amount of CNT volume, the desired stiffness of the FG-CNTRC panels can be achieved. Table 3 also reveals that the SHP panel has the highest value of $\overline{\omega}$ while the HPR panel has the lowest one. This is because HPR has both sagging and hogging curvature along the two directions, neutralizing the effect of each other, while SHP does not. Table 3, once again confirms the accuracy of the present model by comparing the non-dimensional frequencies of the FG-CNTRC plates with the results of Wang [35].

			(0/90)			(0/90) ₂			(0/90) ₄		
Shape	CNT Distribution		V_{CNT}^{*}			V_{CNT}^{*}			V_{CNT}^{*}		
		0.11	0.14	0.17	0.11	0.14	0.17	0.11	0.14	0.17	
	UD	12.854	13.841	15.937	18.658	20.596	23.032	19.829	21.941	24.46	
	FG-A	11.383	12.181	14.177	18.367	20.295	22.700	19.739	21.856	24.37	
CYL	FG-V	12.216	12.969	15.137	18.624	20.528	22.998	19.858	21.963	24.51	
	FG-X	14.442	15.713	17.937	19.084	21.089	23.582	20.062	22.212	24.77	
	FG-O	11.067	11.720	13.753	18.241	20.121	22.528	19.610	21.694	24.20	
	UD	16.540	17.442	20.579	21.334	23.132	26.419	22.364	24.335	27.67	
	FG-A	15.143	15.902	18.967	20.984	22.786	26.050	22.245	24.232	27.58	
SPH	FG-V	16.366	17.080	20.371	21.426	23.193	26.560	22.453	24.423	27.82	
	FG-X	17.812	18.980	22.214	21.722	23.595	26.951	22.587	24.604	28.00	
	FG-O	15.214	15.845	18.990	20.977	22.723	26.008	22.175	24.123	27.47	
	UD	11.295	12.340	13.969	17.583	19.576	21.671	18.806	20.969	23.17	
	FG-A	10.085	10.900	12.474	17.407	19.374	21.459	18.768	20.928	23.13	
HPR	FG-V	10.085	10.900	12.474	17.407	19.374	21.459	18.768	20.928	23.13	
	FG-X	13.057	14.384	16.174	18.023	20.080	22.229	19.042	21.240	23.47	
	FG-O	9.219	9.907	11.409	17.141	19.075	21.128	18.574	20.708	22.88	
Pres	sent UD	11.353	12.401	14.040	17.696	19.701	21.810	18.943	21.122	23.34	
[35	5]	11.348	12.395	14.035	17.714	19.726	21.831	18.958	21.142	23.35	
Pres	sent FG-V	10.138	10.956	12.541	17.519	19.498	21.597	18.906	21.081	23.30	
ATE _[38	5]	10.056	10.876	12.435	17.495	19.484	21.565	18.883	21.065	23.22	
Pres	sent FG-X	13.120	14.453	16.253	18.139	20.208	22.372	19.181	21.395	23.64	
[35	5]	13.064	14.396	16.180	17.975	20.032	22.165	18.995	21.193	23.42	
Pres		9.270	9.960	11.472	17.251	19.197	21.264	18.710	20.859	23.05	
[35	5]	9.182	9.874	11.367	17.378	19.354	21.421	18.856	21.036	23.23	

Table 3. Non-dimensional frequencies $\overline{\omega}$ of the simply supported FG-CNTRC doubly curved shell panels (a/b = 1; R/a = 5, a/h = 50).

3.2.2. Effect of Curvature

Two forms of doubly curved shell panels (SPH and HPR) with a/b = 1, a/h = 20, $R_x = R_y = R_y(0/90)_5$, $V_{CNT}^* = 0.17$ were considered, to study the effect of curvature on the non-dimensional frequencies $\overline{\omega}$. The results are shown in the Figure 3a,b. These figures indicate that at the small value of R/a, the SHP panels have a much higher non-dimensional frequency than HPR panels. The non-dimensional frequencies of the SHP panels decrease, while those of HPR panels increase with the increase of R/a ratio from one to a specific value. After this value, the non-dimensional frequencies of both SHP and HPR panels have approximate values and seem to be unchanged.

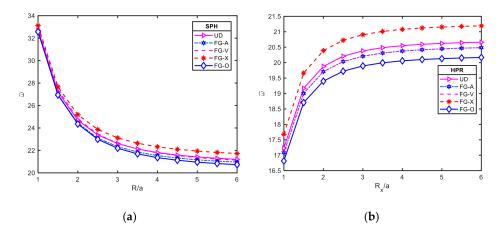


Figure 3. Effect of *R/a* ratio on the frequency parameter $\overline{\omega}$ of FG-CNTRC shell panels. (*a/b* = 1, *a/h* = 20, $R_x = R_y = R$, (0/90)₅, $V_{CNT}^* = 0.17$): (**a**) SPH panel; (**b**) HPR panel.

3.2.3. Effect of Curvature Ratio

The effect of curvature ratio R_x/R_y on non-dimension frequency of the panels is investigated in this subsection. The geometrical dimensions of the panels are taken as a/b = 1, a/h = 20, $R_x/a = 5$. It can be seen from Figure 4a,b, that the non-dimension frequencies of panels decrease with the increase of curvature ratio from -3 to -1, and increase with the value of curvature ratio bigger than -1 for different numbers of layers and different CNT volume fractions. Moreover, the values of $\overline{\omega}$ are at minimum when $R_x/R_y = -1$ shows that the curvature effect can be suppressed if the shell panels have both negative and positive curvature.

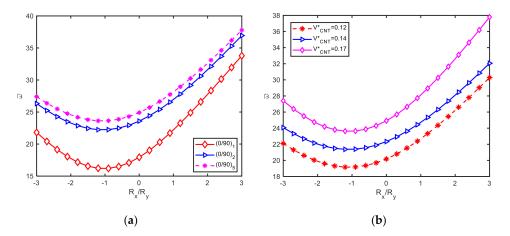


Figure 4. Effect of R_x/R_y of FG-CNTRC shell panels (a/b = 1, a/h = 20, $R_x/a = 5$, FG-X): (**a**) For different number of layers, $V_{CNT}^* = 0.17$; (**b**) for different CNT volume fractions.

3.2.4. Effect of Thickness Ratio

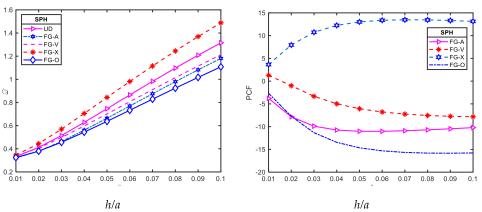
The SPH shell panel was chosen to study the effect of thickness on the free vibration response of the FG-CNTRC doubly curved shell panel. For this purpose, another non-dimensional frequency is defined as [34]:

$$\hat{\omega} = \omega a \sqrt{\frac{\rho^m}{E^m}} \tag{29}$$

$$PCF = \left(\frac{\hat{\omega}_{FG} - \hat{\omega}_{UD}}{\hat{\omega}_{UD}}\right) \times 100 \tag{30}$$

The effect of thickness ratio, h/a, on the non-dimensional frequency of the FG-CNTRC panels is shown in Figure 5. This figure indicates that with all types of CNT distribution, the panels become stiffer with the increase of the thickness ratio, as a result, the non-dimensional frequency of the FG-CNTRC panels increase. Besides, the influence of the thickness ratio, h/a, on the percentage change of frequency (PCF) of the SHP panel is depicted in Figure 5b. It is observed that FG-X panels show positive effectiveness while other FG-CNTRC panels show the negative effects concerning uniformly distribution (UD) panels. The highest percentage change of frequency of an FG-X panel and FG-O panel are about 14.5% and -15.2%, respectively. (3

0.8 0.6



(b)

Figure 5. Effect of h/a ratio on free vibration of FG-CNTRC shell panels ((a/b = 1; $R_x = R_y = R$; $V_{CNT}^* = 0.17$; (0/90)): (a) For the frequency parameter $\hat{\omega} = \omega a \sqrt{\frac{\rho^m}{E^m}}$; (b) for the (PCF).

3.2.5. Effect of Aspect Ratio

(a)

Figure 6a, b show the effects of the aspect ratio (a/b) on the vibration of FG-CNTRC. Here, we take a/b = 1; $R_x = R_y = R$; R/a = 5; $V_{CNT}^* = 0.17$ and (0/90).

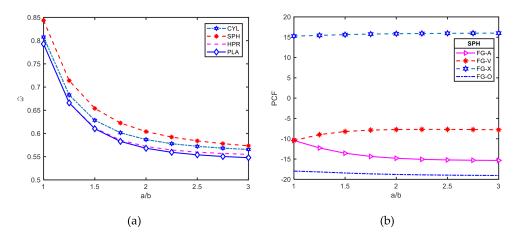


Figure 6. Effect of aspect ratio (*a/b*) on free vibration of FG-CNTRC shell panels (a/b = 1; $R_x = R_y = R$; $V_{CNT}^* = 0.17$; (0/90)): (a) For the frequency parameter $\hat{\omega} = \omega a \sqrt{\frac{\rho^m}{E^m}}$; (b) For the percentage change of frequency (PCF).

Figure 6a reveals that the non-dimensional frequencies of all four types of doubly curved panels decrease uniformly by increasing aspect ratio. In other words, the stiffness of doubly curved panels will be reduced as the aspect ratio increases. Figure 6a states that the PCF of the FG-CNTRC panels remains unchanged with the increase of aspect ratio.

3.2.6. Effect of Number of Layers

The influence of number of layers (*n* is a couple of layers (0/90)) on $\hat{\omega}$, and PCF are depicted in Figure 7a,b, respectively. Here, the geometrical dimensions of the panels are taken as a/b = 1, a/h = 20, $R_x = R_y = R$, and $V_{CNT}^* = 0.17$, FG-X, (0/90)_n. As the Figures show, with a fixed value of total thickness, the non-dimensional frequencies and the percentage change of frequency of laminated FG-CNTRC panels are strongly affected by the number of layers, changing from one layer to two layers. However, these two dimensionless parameters vary very slightly for the number of layers greater than three. This is compatible with the investigations of Reddy [31], for conventional fiber reinforced composites.

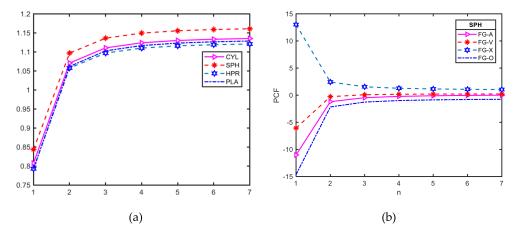


Figure 7. Effect of number of layers (n) on free vibration of FG-CNTRC shell panels (a/b = 1, $R_x = R_y = R$; $V_{CNT}^* = 0.17$, (0/90)_n: (**a**) For the frequency parameter $\hat{\omega} = \omega a \sqrt{\frac{\rho^m}{E^m}}$; (**b**) for the percentage change of frequency *PCF*.

3.2.7. Effect of Different Wave Numbers

Table 4 listed non-dimensional frequencies for two-layered (0/90) FG-CNTRC doubly curved shell panels (a/b = 1; R/a = 5, a/h = 50, FG-X, $V_{CNT}^* = 0.17$) for different wave numbers. It can be seen that at the small value of wave numbers (n, m) the SPH panels have highest non-dimensional frequencies while the HPR panels have lowest ones. However, it also can be seen that the non-dimensional frequencies of all three types of doubly curved panels will approximately have more wave numbers.

Shape	n	<i>m</i> = 1	<i>m</i> = 2	<i>m</i> = 3	<i>m</i> = 4	<i>m</i> = 5	<i>m</i> = 6
	1	17.937	48.140	99.846	169.706	254.950	352.930
	2	46.027	64.594	109.402	176.466	260.683	358.380
CYL	3	97.820	109.032	141.357	198.901	277.607	372.340
CIL	4	168.004	176.095	198.835	243.742	311.861	399.381
	5	253.606	260.401	277.580	311.892	367.569	444.182
	6	351.923	358.209	372.384	399.478	444.253	508.835
	1	22.214	49.271	100.122	169.593	254.585	352.371
	2	49.472	65.771	109.763	176.410	260.356	357.845
SPH	3	100.573	109.960	141.685	198.853	277.285	371.804
SFH	4	170.323	176.802	199.065	243.665	311.532	398.838
	5	255.596	260.940	277.698	311.759	367.219	443.632
	6	353.644	358.612	372.397	399.275	443.867	508.271
	1	16.174	47.141	99.497	169.711	255.202	353.378
	2	47.141	64.179	109.145	176.475	260.924	358.816
HPR	3	99.497	109.145	141.314	198.990	277.883	372.799
пгк	4	169.711	176.475	198.990	243.935	312.190	399.872
	5	255.202	260.924	277.883	312.190	367.960	444.710
	6	353.378	358.816	372.799	399.872	444.710	509.403

Table 4. Non-dimensional frequencies \overline{w} for two-layered (0/90) FG-CNTRC doubly curved shell panels for different wave numbers (a/b = 1; R/a = 5, a/h = 50, , FG-X, $V_{CNT}^* = 0.17$).

Figures 8–10 depict the first six mode shapes of the simply supported laminated FG-CNTRC CYL, SPH and HPR shell panels, respectively. Geometric characteristics of the panels are a/b = 1, R/a = 5 and a/h = 50. Type of CNT distribution is FG-X and volume faction of CNT is $V_{CNT}^* = 0.17$. It can be noticed from these Figures, that in CYL panels, mode (m = 2, n = 1) is higher than mode (m = 1, n = 2), while in SPH and HPR panels, mode (m = 1, n = 2) and mode (m = 2, n = 1) are the same order. This is because the CYL panel only has the curvature in x direction while SPH and HPR panels have the curvature

in both *x* and *y* directions. These mode shapes can help to understand vibration characteristics of laminated FG-CNTRC doubly curved shell panels.

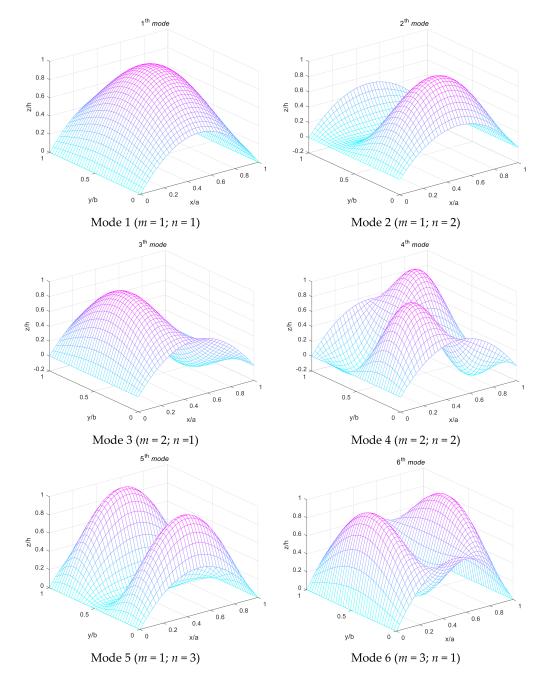


Figure 8. The first six mode shapes of simply supported laminated FG-CNTRC CYL panels.

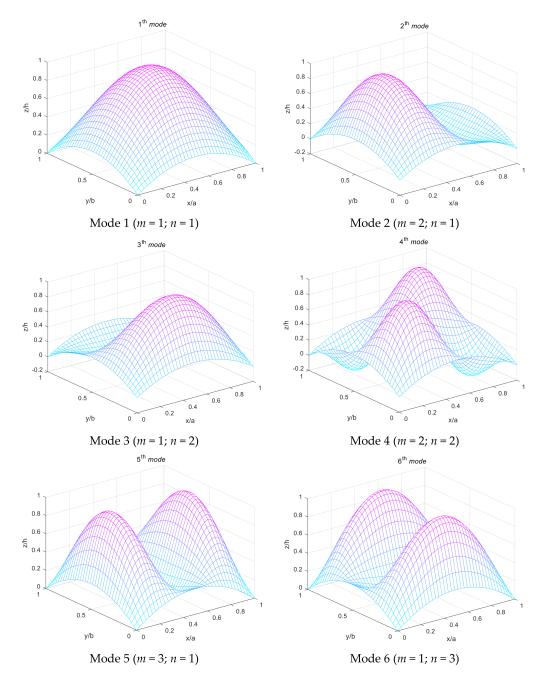


Figure 9. The first six mode shapes of simply supported laminated FG-CNTRC SPH panels.

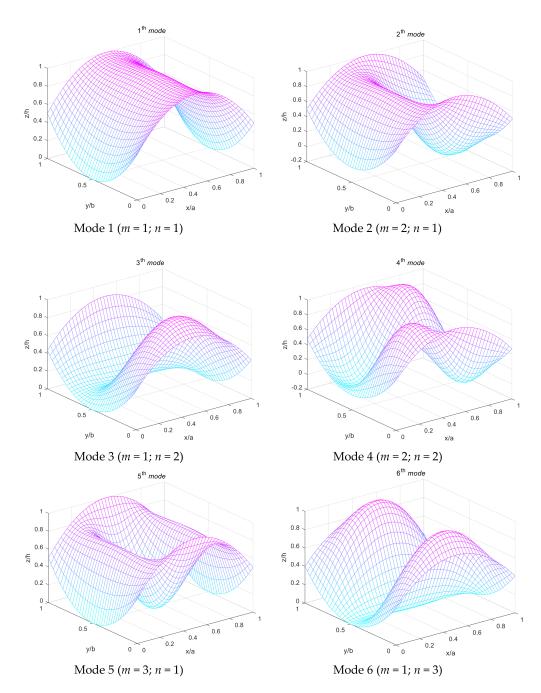


Figure 10. The first six mode shapes of simply supported laminated FG-CNTRC HPR panels.

4. Conclusions

In this paper, an analytical solution based on a new four-variable refined shell theory for free vibration analysis of the laminated FG-CNTRC doubly curved shell panels was developed. The accuracy and efficiency of the present model are validated through a review of comparison studies. The influences of several parameters such as FG-CNTRC parameters, curvature, curvature ratio, thickness ratio, aspect ratio and the number of layers on free vibration responses of the panels are explored. The results revealed that the shell panels become stiffer with increasing curvature, conversely, the stiffness of the panels is reduced as the aspect ratio increases. FG-X CNTRC panels have the highest frequency, while FG-O CNTRC panels have the smallest frequency regarding all inlet studied parameters.

The present theory is accurate and efficient in solving free vibration behaviours of doubly curved laminated FG-CNT reinforced composite panels and may be useful in the study of similar composite structures.

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Appendix A

Detailed steps to construct the new shape function:

The transverse strains associated with the displacement field in Equation (1) are:

$$\begin{aligned} \gamma_{xz} &= \frac{1}{1+z/R_x} \begin{bmatrix} \frac{\partial w}{\partial x} + \frac{\partial u}{\partial z} - \frac{u_0}{R_x} \end{bmatrix} = \frac{1}{1+z/R_x} \begin{bmatrix} \frac{\partial w_b}{\partial x} + \frac{\partial w_s}{\partial x} + \frac{u_0}{R_x} - \frac{\partial w_b}{\partial x} - f'(z) \frac{\partial w_s}{\partial x} - \frac{u_0}{R_x} \end{bmatrix} \\ &= \frac{1}{1+z/R_y} \begin{bmatrix} (1-f'(z)) \frac{\partial w_s}{\partial x} \end{bmatrix} \\ \gamma_{yz} &= \frac{1}{1+z/R_y} \begin{bmatrix} \frac{\partial w}{\partial y} + \frac{\partial v}{\partial z} - \frac{v_0}{R_y} \end{bmatrix} = \frac{1}{1+z/R_y} \begin{bmatrix} \frac{\partial w_b}{\partial y} + \frac{\partial w_s}{\partial y} + \frac{v_0}{R_y} - \frac{\partial w_b}{\partial y} - f'(z) \frac{\partial w_s}{\partial y} - \frac{v_0}{R_y} \end{bmatrix} \end{aligned}$$
(A1)
$$&= \frac{1}{1+z/R_y} \begin{bmatrix} (1-f'(z)) \frac{\partial w_s}{\partial y} \end{bmatrix}$$

For shells under bending, the transverse shear stresses σ_{xz} , σ_{yz} must be vanished at the top and bottom surfaces. These conditions lead to the requirement that the corresponding transverse strains on these surfaces have to be zero. From $\gamma_{xz}(x, y, \pm \frac{h}{2}) = \gamma_{yz}(x, y, \pm \frac{h}{2}) = 0$, we obtain:

$$\gamma_{xz} = \frac{1}{1 + z/R_x} \left[(1 - f'(z)) \frac{\partial w_s}{\partial x} \right] = 0 \quad at \ z = \pm \frac{h}{2}$$
(A2)

$$\gamma_{yz} = \frac{1}{1 + z/R_y} \left[(1 - f'(z)) \frac{\partial w_s}{\partial y} \right] = 0 \quad at \ z = \pm \frac{h}{2}$$
(A3)

From Equations (A2) and (A3), we have:

$$f'(z) = 1 at z = \pm \frac{h}{2}$$
 (A4)

Function f(z) satisfies the condition (5) can be selected as a polynomial, trigonometric, and exponential, ... function. In our study, we chose f(z) as a cubic polynomial: $f(z) = az + \frac{b}{h^2}z^3$, thus:

$$f'(z) = a + \frac{3b}{h^2}z^2 = 1$$
(A5)

Some authors have chosen the value of the pair a, b to satisfy Equation (A5). In this study, we chose: a = -1/8, b = 3/2. Thus:

$$f(z) = -\frac{1}{8}z + \frac{3}{2}\frac{z^3}{h^2}, \ f'(z) = \left(-\frac{1}{8} + \frac{3.3}{2}\frac{z^2}{h^2}\right)\Big|_{z=\pm\frac{h}{2}} = 1$$
(A6)

Appendix B

Matrix elements of Equation (25):

$$s_{11} = -\left(\overline{A}_{11} + 2\frac{\overline{B}_{11}}{R_x} + \frac{\overline{D}_{11}}{R_x^2}\right) \alpha_m^2 - \left(\hat{A}_{66} + 2\frac{\hat{B}_{66}}{R_x} + \frac{\hat{D}_{66}}{R_x^2}\right) \beta_n^2 \tag{A7}$$

$$s_{12} = -\left(A_{12} + A_{66} + (B_{12} + B_{66})\left(\frac{1}{R_x} + \frac{1}{R_y}\right) + \frac{1}{R_x R_y}(D_{12} + D_{66})\right)\beta_n \alpha_m$$
(A8)

$$s_{14} = \left(\overline{B}_{11}^{s} + \frac{\overline{D}_{11}^{s}}{R_x}\right) \alpha_m^3 + \left(\left(B_{12}^{s} + B_{66}^{s} + \hat{B}_{66}^{s} + \frac{D_{66}^{s}}{R_x} + \frac{D_{66}^{s}}{R_x} + \frac{D_{12}^{s}}{R_x}\right) \beta_n^2 + \frac{\overline{A}_{11}}{R_x} + \frac{A_{12}}{R_y} + \frac{\overline{B}_{11}}{R_x^2} + \frac{B_{12}}{R_x R_y}\right) \alpha_m \tag{A9}$$

$$s_{22} = -\left(\overline{A}_{66} + 2\frac{\overline{B}_{66}}{R_y} + \frac{\overline{D}_{66}}{R_y^2}\right) \alpha_m^2 - \left(\hat{A}_{22} + 2\frac{\hat{B}_{22}}{R_y} + \frac{\hat{D}_{22}}{R_y^2}\right) \beta_n^2$$
(A10)

$$s_{23} = \left(\frac{A_{12}}{R_x} + \frac{\hat{A}_{22}}{R_y} + \frac{1}{R_y} \left(\frac{B_{12}}{R_x} + \frac{\hat{B}_{22}}{R_y}\right)\right) \beta_n + \left(\hat{B}_{22} + \frac{\hat{D}_{22}}{R_y}\right) \beta_n^3 + \left(B_{12} + B_{66} + \overline{B}_{66} + \frac{1}{R_y} \left(D_{12} + D_{66} + \overline{D}_{66}\right)\right) \beta_n \alpha_m^2$$
(A11)

$$s_{24} = \left(\frac{A_{12}}{R_x} + \frac{\hat{A}_{22}}{R_y} + \frac{1}{R_y} \left(\frac{B_{12}}{R_x} + \frac{\hat{B}_{22}}{R_y}\right)\right) \beta_n + \left(\hat{B}_{22}^s + \frac{\hat{D}_{22}^s}{R_y}\right) \beta_n^3 + \left(B_{12}^s + B_{66}^s + \overline{B}_{66}^s + \frac{1}{R_y} \left(D_{12}^s + D_{66}^s + \overline{D}_{66}^s\right)\right) \beta_n \alpha_m^2$$
(A12)

$$s_{33} = -\frac{\overline{A}_{11}}{R_x^2} - 2\frac{A_{12}}{R_x R_y} - \frac{\hat{A}_{22}}{R_y^2} - 2\left(\frac{B_{12}}{R_x} + \frac{\hat{B}_{22}}{R_y}\right)\beta_n^2 - 2\left(\frac{\overline{B}_{11}}{R_x} + \frac{B_{12}}{R_y}\right)\alpha_m^2 - \overline{D}_{11}\alpha_m^4 - \left(2D_{12} + 2D_{66} + \overline{D}_{66} + \hat{D}_{66}\right)\alpha_m^2\beta_n^2 - \hat{D}_{22}\beta_n^4$$
(A13)

$$s_{34} = -\frac{\overline{A}_{11}}{R_x^2} - 2\frac{A_{12}}{R_x R_y} - \frac{\hat{A}_{22}}{R_y^2} - \left(\frac{\overline{B}_{11}}{R_x} + \frac{\overline{B}_{11}^s}{R_x} + \frac{B_{12}}{R_y} + \frac{B_{12}^s}{R_y}\right) \alpha_m^2 - \left(\frac{B_{12}}{R_x} + \frac{B_{12}^s}{R_x} + \frac{\hat{B}_{22}}{R_y} + \frac{\hat{B}_{22}}{R_y}\right) \beta_n^2 - \left(2D_{12}^s + 2D_{66}^s + \overline{D}_{66}^s\right) \alpha_m^2 \beta_n^2 - \overline{D}_{11}^s \alpha_m^4 - \hat{D}_{22}^s \beta_n^4$$
(A14)

$$s_{13} = \left(\frac{\overline{A}_{11}}{R_x} + \frac{A_{12}}{R_y} + \frac{1}{R_x} \left(\frac{\overline{B}_{11}}{R_x} + \frac{B_{12}}{R_y}\right) \left(B_{12} + B_{66} + \hat{B}_{66} + \frac{1}{R_x} \left(D_{12} + D_{66} + \hat{D}_{66}\right)\right) \beta_n^2 \alpha_m + \left(\overline{B}_{11} + \frac{\overline{D}_{11}}{R_x}\right) \alpha_m^3$$
(A15)

$$s_{44} = -\frac{\overline{A}_{11}}{R_x^2} - 2\frac{A_{12}}{R_x R_y} - \frac{\hat{A}_{22}}{R_y^2} - \left(\hat{A}_{44}^s + 2\frac{B_{12}^s}{R_x} + 2\frac{\hat{B}_{22}}{R_y}\right)\beta_n^2 - \left(\overline{A}_{55}^s + 2\frac{\overline{B}_{11}^s}{R_x} + 2\frac{B_{12}^s}{R_y}\right)\alpha_m^2 - \left(2E_{12}^s + 2E_{66}^s + \overline{E}_{66}^s + \hat{E}_{66}^s\right)\alpha_m^2\beta_n^2 - \overline{E}_{11}^s\alpha_m^4 - \hat{E}_{22}^s\beta_n^4$$
(A16)

$$m_{11} = -\left(\bar{I}_0 + 2\frac{\bar{I}_1}{R_x} + \frac{\bar{I}_2}{R_x^2}\right); m_{12} = 0; m_{13} = \left(\bar{I}_1 + \frac{\bar{I}_2}{R_x}\right) \alpha_m; m_{14} = \left(\bar{J}_1 + \frac{\bar{J}_2}{R_x}\right) \alpha_m; m_{11} = -\left(\bar{I}_0 + 2\frac{\bar{I}_1}{R_y} + \frac{\bar{I}_2}{R_y^2}\right); m_{23} = \left(\bar{I}_1 + \frac{\bar{I}_2}{R_y}\right) \beta_n; m_{24} = \left(\bar{J}_1 + \frac{\bar{J}_2}{R_y}\right) \beta_n; m_{33} = -\bar{I}_0 - \bar{I}_2 \left(\alpha_m^2 \beta_n^2\right); m_{34} = -\bar{I}_0 - \bar{I}_2 \left(\alpha_m^2 \beta_n^2\right); m_{44} = -\bar{I}_0 - \bar{K}_1 \left(\alpha_m^2 \beta_n^2\right); m_{44} = -\bar{I}_0 - \bar{K}_1$$

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