

# Modelling Variable Speed Pumps for Flow and Pressure Control Using Nash Equilibrium <sup>†</sup>

Jochen W. Deuerlein <sup>1,2,\*</sup> , Sylvan Elhay <sup>3</sup> , Olivier Piller <sup>2,4</sup> , Michael Fischer <sup>1</sup> and Angus R. Simpson <sup>2</sup> 

<sup>1</sup> 3S Consult GmbH, 30827 Garbsen, Germany; fischer@3sconsult.de

<sup>2</sup> School of Architecture and Civil Engineering, University of Adelaide, Adelaide, SA 5005, Australia; olivier.piller@inrae.fr (O.P.); angus.simpson@adelaide.edu.au (A.R.S.)

<sup>3</sup> School of Computer and Mathematical Sciences, University of Adelaide, Adelaide, SA 5005, Australia; sylvan.elhay@adelaide.edu.au

<sup>4</sup> INRAE, AQUA Division, UR ETTIS, F-33612 Cestas, France

\* Correspondence: deuerlein@3sconsult.de; Tel.: +49-721-20397-521

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**Abstract:** Recently, the Nash equilibrium, known from game theory, was used for steady state calculation of pressurized pipe systems with general flow and pressure control devices. The concept is now applied to pumping stations. It is assumed that at least one of the pumps has a frequency controller that enables the pump to deliver a given set flow or set pressure by adaptation of pump speed. For hydraulic calculation, the system is decomposed into the local pumping station and a surrogate link with flow or pressure constraints at one of its end nodes. This method is demonstrated for a small example system.

**Keywords:** pump control; inverse problem; Nash equilibrium; content model; variable speed pump



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## 1. Introduction

Pumps are indispensable components for the transport of fluids in pressurized pipe systems. The hydraulic behaviour of a fixed-speed pump (FSP) is usually modelled by its characteristic curve, which may be adapted for variable speed pumps (VSPs) in hydraulic simulation software, such as EPANET. In both cases, the operation of the pump is determined by a given relation between pump flow and pumping head. However, in real systems, the VSP is combined with a controller that allows the adjustment of the pump speed to reach a desired pump flow or downstream head. In this case, the rpm (revolutions per minute) of the pump is unknown and depends on the system hydraulics. Mathematical modelling of flow or pressure-controlled pumps is not as straightforward as in the case of FSPs. Instead, determining the pump speed that is required to reach a certain flow or pressure is an inverse problem that is more difficult to solve.

Recently, a mathematical framework for the simulation of systems with flow and pressure regulating valves has been published [1–3]. The general approach was based on the Nash equilibrium of the flow-constrained minimization of the Content function and local optimization problems for pressure control valves. In this paper, we extend this approach to also model flow and pressure-controlled pumps in water systems. For flow control, a minimum flow bound is introduced. In contrast to how flow control valves (FCVs) are usually used, here, the minimum flow bound has a positive sign. Consequently, the interpretation of the Lagrange multiplier of the active flow constraint is not head loss, but rather the pumping head.

Rather, the modelling of the VSP, which controls the downstream pressure, follows by adaptation of the authors' Nash equilibrium scheme for pressure regulating valves. In this case, the sign of the variable  $z$ , which refers to the head loss for valves, is inverted.

A negative  $z$  can be interpreted as the head gain that is to be delivered by the pump to reach the required downstream pressure. In both cases, the Lagrange multipliers determine the operational state of the pump. A strength of this approach is that there is no need for heuristics. The paper demonstrates the adaptation of the Nash equilibrium approach to pump control. The method is illustrated with a small example.

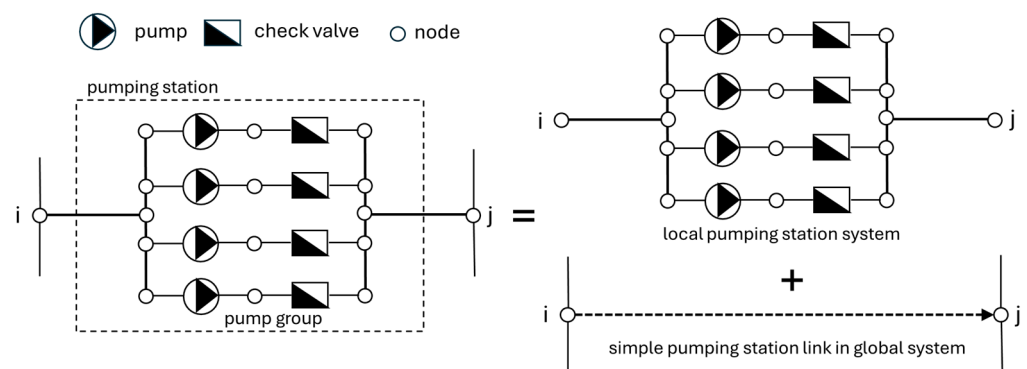
## 2. Methods

### 2.1. Modelling of Pumping Stations

Pumps are essential components of water distribution systems. Normally, pumping stations do not consist of a single pump, but multiple pumps that are combined in parallel and/or in series. Such pump groups allow for improved operational flexibility and efficiency as well as increased system reliability, since in the case that a pump fails, it can be replaced by another stand-by pump. The pump characteristics of serial and parallel pumps are well known. If the pumps are in parallel, their volume flow is added, whereas for pumps in series, their pumping heads are added.

Often one (or more) of the pumps is equipped with a frequency controller that enables the pump to respond to changing operational conditions in the system by adapting the pump speed (rpm) to reach the optimal efficiency. Such frequency controllers also allow the stepless control of the required pumping flow, pumping head or system pressure at a certain location where the control node can be directly downstream of the pumping station or at a distance. In real systems, combinations of frequency-controlled pumps and simple pumps can be found. Strictly speaking, the calculation of the required rpm for flow or pressure controlled pumping stations is an inverse problem formulated as a hierarchical optimization problem. These circumstances in combination make the stable calculations of general pumping stations a challenging task.

The basic idea is to decompose the problem into the global hydraulic network simulation and local pumping station calculations (see Figure 1). The steps are as follows: First, the pumping station is replaced by a simple link with unknown flow-head gain characteristics. In addition, a set value for this link (flow) or the downstream node head of the pumping station is defined. Once the solution for the global system has converged, within the local calculation step, the combination of pumps and their rpm, for which the consumption of electrical energy is minimal, is determined. Other criteria for the local calculation are possible as well; for example, by defining which pumps are required to be on stand-by in order to minimize their deterioration.



**Figure 1.** Decomposition of system with a pumping station.

### 2.2. Mathematical Model

Recently, a comprehensive mathematical formulation for the calculation of the steady state of pressure-dependent water supply networks including pressure and flow regulating devices has been developed. In this approach, the Nash equilibrium of multiple constrained nonlinear optimization problems is calculated. In this context, the Content minimization for flow-controlled systems with pressure dependent demands is augmented by individual

local optimization problems for each pressure control device (PRV: Pressure Reducing Valve, PSV: Pressure Sustaining Valve). Benefits of this approach include that it fully covers different operational states of the control devices and that conditions for existence and uniqueness of the hydraulic steady state can be derived from the rigorous mathematical model that does not rely on any heuristics. The Nash equilibrium of the following convex optimization problems is calculated as

$$\min_{\mathbf{q}, \mathbf{c} \in \mathbf{B}} C(\mathbf{z}; \mathbf{q}, \mathbf{c}) = \sum_{j=1}^{n_p} \int_0^{q_j} \xi_j(s) ds - \mathbf{a}^T \mathbf{q} + \mathbf{c}^T (\mathbf{u} + h_m) + \sum_{i=1}^{n_j} (h_s - h_m) \int_0^{c_i} \gamma^{-1} \left( \frac{s}{d_i} \right) ds + \mathbf{z}^T \mathbf{q} \quad (1)$$

$$\min_{z_k \geq 0} f(\mathbf{q}, \mathbf{c}; z_k) = \frac{1}{2} \left( h_{i_k}(\mathbf{q}, \mathbf{c}) - \xi(q_k) - z_k - h_{j_k}^S \right)^2, \quad \forall k \in I_z \quad (2)$$

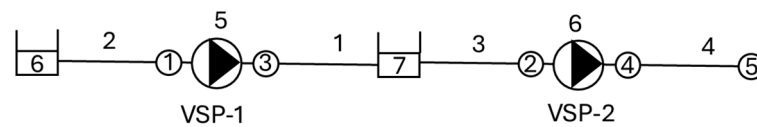
The system has  $n_p$  links and  $n_j$  junctions,  $C$  is the Content function,  $\mathbf{q}$  is the pipe flow vector and  $\mathbf{c}$  is the external flow vector of demand nodes.  $\mathbf{B}$  represents the polyhedron defined by the continuity equation combined with the box constraints for  $\mathbf{q}$  and  $\mathbf{c}$ . Vector  $\mathbf{u}$  denotes the geodetic elevation, and  $h_m$  and  $h_s$  are the minimum head and the minimum supply head, respectively. The nonlinear functions  $\xi$  and  $\gamma$  refer to the frictional head loss and the pressure outflow relationship function (POR), respectively, and  $\mathbf{z}$  denotes the unknown head loss of pressure control devices. There is mutual feedback between the minimization problems. Whereas, in Content minimization, the unknown head losses are assumed to have constant values, and the decision variables are the flows  $\mathbf{q}$  and  $\mathbf{c}$ ; in the pressure control problem, the flows are assumed to be fixed and the decision variables are the head losses of the control devices. The Nash equilibrium (from mathematical game theory) of the various mutually combined optimization problems provides the solution [1].

The extension of the Nash equilibrium approach to the application for variable speed pumps is relatively straightforward. For pumping stations with flow control, a minimum set flow for the link is added to the box constraints in Equation (1). In this case, the minimum flow bound is positive. Consequently, the Lagrange multiplier represents a negative head loss in the positive link direction which is exactly the pumping head that is required to deliver the set flow. The pumping station with flow control can be modelled by Content minimization [2,3] (Equation (1)) solely. In contrast, the second control mode for maintaining a given set pressure at the discharge side of the pump station requires an additional minimization problem, according to Equation (2). The situation is similar to the one with pressure control devices that adapt the head loss in order to keep the upstream pressure (PSV) or downstream pressure (PRV) constant [1]. The only difference is that the  $z$  value must be negative (pressure gain).

After convergence of the system equations, the speed, the flow and the pumping head of the individual pumps of the local pumping stations are calculated. Besides the given set flow and pressure, additional constraints can be added to the system equations that refer, for example, to the maximum capacity of the pumping station.

### 3. Example Network

The system in Figure 2 shows how a link with a positive lower flow bound,  $q_{min} > 0$ , can model a pump, and a link with head gain,  $z < 0$  can model a VSP. Each link has a diameter of 0.5 m, and pipe roughness 0.25 mm. Nodes 6 and 7 have elevations of 10 m and 200 m, respectively. Links 5 and 6 have lengths of 1 m, and all other links have lengths of 1000 m. The service pressure head is 20 m, and the minimum pressure head is 0 m. The head loss is modelled by the Darcy–Weisbach formula, and the POR is the 1-side regularized Wagner function [3]. The flow in Link 5,  $q_5$ , is constrained to  $300 < q_5 < 600$  L/s and Node 4, downstream of Link 6, has  $z < 0$  and its set point is 290 m.



**Figure 2.** Example system with two VSPs: VSP-1 with flow control, and VSP-2 with pressure control.

The solution was found in 7 iterations of the authors' Matlab code; it has a delivery fraction of 75.9% (node 2:  $c = 107.59$  L/s,  $d = 200$  L/s, node 3:  $36.04$  L/s,  $d = 40$  L/s, node 5:  $c = 342.05$  L/s,  $d = 400$  L/s) and the set point of 290 m (node 4) is achieved by a pumping head of  $z = -99.2$  m. The heads at Nodes 1 (5.85 m) and 3 (203.23 m) indicate to a designer that the head gain that the pump in Link 5 would be required to provide is the value of the Lagrange multiplier of the minimum flow bound (300 L/s); for that link,  $\kappa = 197.4$  m. The full data set can be obtained from the authors upon request.

#### 4. Discussion and Conclusions

The methodology presented here is particularly suited for optimal design and operation of systems including pumping stations. In the first case, where the pumping station does not yet exist, the method allows the simulation of pipe systems with consideration of design flows and/or pressures of the pumping station. In the second case, the local pumping station optimization could be combined with a global optimizer for the pipe system. The method has been used for the formation of a hierarchical optimization problem for optimal pump scheduling in a system with multiple storage tanks and pumping stations. The decision variables of the upper optimization problem formulation are the pumping flows. The local pumping station optimization of the lower level calculates the combination of pumps and their speeds (if a frequency controller is available) that requires minimum electrical power. The objective function of the upper level includes the total operational cost.

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